

**APPENDIX A**

Preliminary Geotechnical Report

**PRELIMINARY GEOTECHNICAL REPORT  
PROPOSED COUNTY OF RIVERSIDE  
HEMET REGIONAL SERVICE CENTER  
PARKING LOT IMPROVEMENT PROJECT  
723 AND 749 NORTH STATE STREET  
HEMET, CALIFORNIA  
A.P.N. 439-060-013**

**PREPARED FOR:**

**COZAD & FOX, INC.**  
Attention: Brian Fox, P.E.  
151 South Girard Street  
Hemet, California 92544

**PREPARED BY:**

**INLAND FOUNDATION ENGINEERING, INC.**  
1310 South Santa Fe Avenue  
San Jacinto, California 92583

October 7, 2014  
Project No. C143-050

**INLAND FOUNDATION ENGINEERING, INC.**  
*Consulting Geotechnical Engineers and Geologists*  
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October 7, 2014  
Project No. C143-050

Attention: Brian Fox, P.E.  
**COZAD & FOX, INC.**  
151 South Girard Street  
Hemet, California 92544

Re: Preliminary Geotechnical Report  
Proposed County of Riverside Hemet Regional Service Center  
Parking Lot Improvement Project  
723 and 749 N. State Street, Hemet, California  
A.P.N. 439-060-013

Dear Mr. Fox:

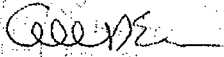
We are pleased to submit this preliminary geotechnical report prepared for the referenced project. The site is located at 723 and 749 N. State Street in the City of Hemet, California.

It is our opinion that the proposed development is feasible from a geotechnical engineering standpoint. Our report includes design recommendations along with the field and laboratory data. We have also included recommendations for site grading.

We appreciate being of service to you on this project. If you have any questions, please contact our office.

Respectfully,

**INLAND FOUNDATION ENGINEERING, INC.**

  
**Allen D. Evans, President**  
R.C.E. 38104/G.E. 2060

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## **INTRODUCTION**

This report presents the results of the preliminary geotechnical investigation conducted at the site of the County of Riverside Hemet Regional Service Center Parking Lot Improvement Project. The proposed parking lot is to be located at 723 and 749 North State Street in the City of Hemet, California. The following was used as a reference during our investigation:

- A plan entitled "Precise Grading Plan for County of Riverside Hemet Regional Service Center Parking Lot", dated July 2014 and prepared by Cozad & Fox, Inc.

This report provides preliminary design parameters that may be applied to the proposed site improvements.

## **SCOPE OF SERVICES**

The purpose of this geotechnical investigation was to provide geotechnical engineering parameters for design and construction of the proposed project. The scope of our services included:

- *A review of the general subsurface conditions at the project site.*
- *An evaluation of the engineering data collected for the project site.*
- *Preparation of this report providing preliminary geotechnical engineering conclusions and recommendations for design and construction.*

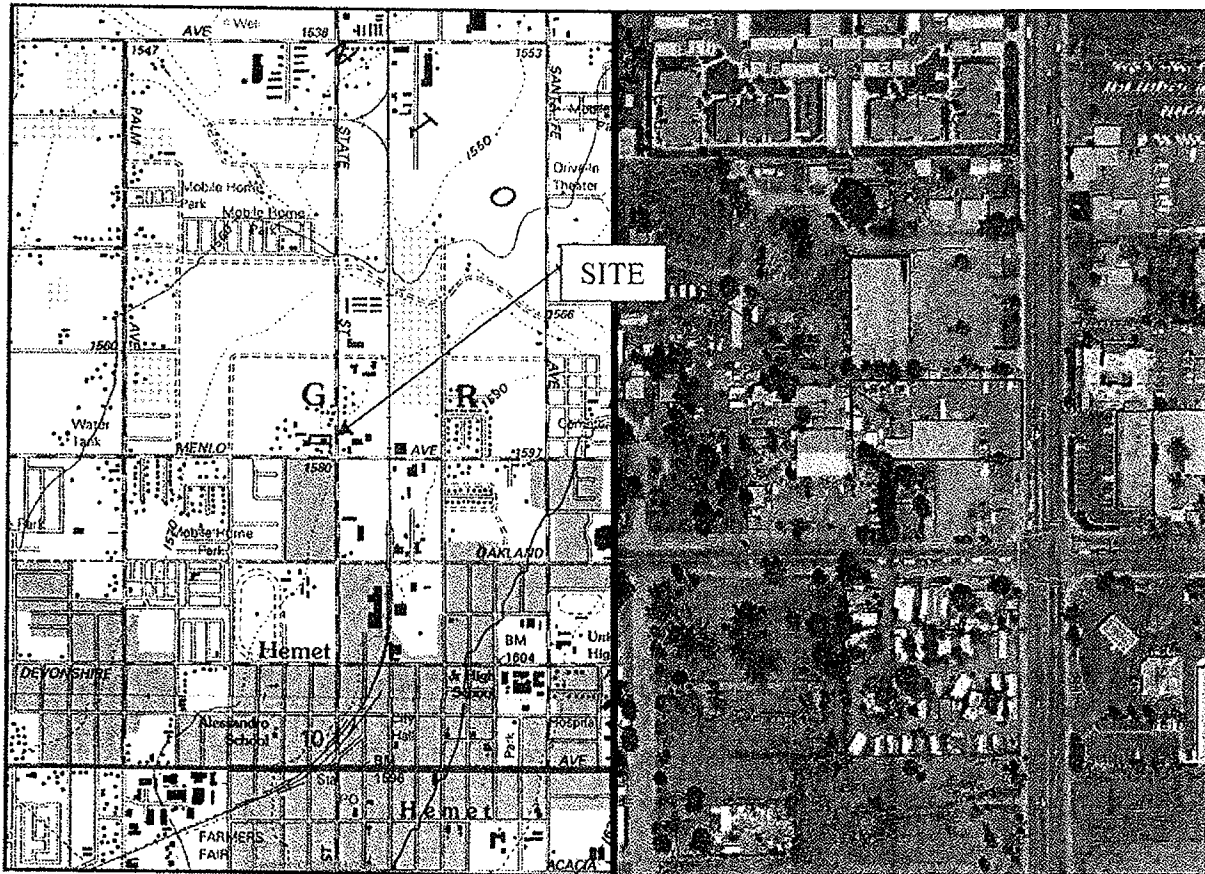
The tasks performed to achieve these objectives included:

- *Subsurface exploration to evaluate the nature and stratigraphy of the subsurface soils and to obtain representative samples for laboratory testing.*
- *Laboratory testing of representative samples to evaluate the classification and engineering properties of the soils.*
- *Analysis of the data collected and the preparation of this report with preliminary geotechnical engineering conclusions and recommendations.*

Evaluation of hazardous waste or seismic hazards was not within the scope of services provided. The information in this report represents professional opinions that have been developed using that degree of care and skill ordinarily exercised, under similar circumstances, by reputable geotechnical consultants practicing in this or similar localities. No other warranty, either expressed or implied, is made as to the professional advice included in this report.

## PROJECT DESCRIPTION

The site rests in the southerly portion of Section 3, Township 5 South, Range 1 West, S.B.B.&M. The subject site rests north and west of the intersection of State Street and Menlo Avenue in the City of Hemet, California. The site is located in a mixed usage area of Hemet. The site consists of approximately 0.92 acres and is bounded on the east by State Street, west by vacant land, north by the existing Hemet Regional Service Center, and south by unoccupied residences and vacant land.



U.S.G.S. Topographic Map, San Jacinto 7.5' Quadrangle, and Aerial Photograph (2010)

At the present time, the site is vacant. The topography may be described as planar. The grading plan indicates an existing elevation of 1581' above mean sea level (msl) near the southwest corner of the property. The central and northern portions of the site rest at an approximate elevation of 1582' above msl. A moderately dense growth of seasonal weeds was present on the site at the time of our field investigation. Previous structures and pavement on the site were recently demolished and cleared. The aerial image above indicates the locations of previous structures on the site.

The proposed construction is to consist of a paved parking lot comprising approximately 39,000 square feet. A six foot high security fence is planned for the perimeter of the parking lot. A 3'-4" high retaining wall is planned along a portion of the southerly property line. Two self-retaining retention basins are also planned along the southerly portion of the site.

Grading is expected to consist of cuts and fills of less than two feet, with the exception of remedial grading as recommended in this report.



## ***SUBSURFACE CONDITIONS***

The field and laboratory exploration and testing indicate that the site is underlain by alluvial deposits that are generally in a loose to medium dense condition to the depth explored. The native soils encountered consist of predominately fine- to medium-grained silty sands and poorly graded sands.

A surficial veneer of uncontrolled artificial fill and disturbed soils is present across most of the site. Within our exploratory borings, up to two feet of artificial fill was encountered. Based on the presence of former structures and previous grading on the site, areas of deeper artificial fill and debris may be present on portions of the site. This may include construction debris, abandoned utility lines and miscellaneous areas of buried fill. Where encountered in our borings, these materials consisted of silty sands with gravel, asphalt, and concrete fragments.

Laboratory testing indicates native soils within the zone of influence to the proposed development are non-plastic and should be assumed to be non-expansive.

Analytical testing indicates the concentration of sulfates in the soil is negligible with respect to sulfate attack on concrete. Chloride concentrations are less than 500 parts per million. The soil is neutral to slightly alkaline with a pH value of 7.8. The saturated resistivity value of 13,780 ohm-cm indicated that the soil is not highly corrosive to buried metal.

## CONCLUSIONS AND RECOMMENDATIONS

On the basis of our field and laboratory exploration and testing, it is our opinion that the proposed parking lot is feasible from a geotechnical engineering standpoint. The presence of existing fill will require the removal and recompaction of soil across most of the site. It is estimated that most of this removal and recompaction will be in the southerly and westerly portions of the site, associated with the former presence and demolition of structures on the site.

Our testing indicates that on-site soils may be assumed to be non-expansive. Analytical testing indicates sulfates concentrations are very low. Per ACI 318, Table 4.2.1, the soil can be classified as Class S0 with respect to sulfate exposure. Testing indicates that severely corrosive soils are not expected to be present on the site. Chloride concentrations are also very low.

The following paragraphs present more detailed design criteria which have been developed on the basis of our field and laboratory exploration and testing.

**Tentative Pavement Design:** All surfaces to receive asphalt concrete paving should be underlain by a minimum compacted fill thickness of 12 inches (excluding aggregate base). This should be performed as described in the Site Grading Section of this report. On the basis of a preliminary R-value of 56, we make the following tentative recommendations for structural pavement section design:

Service	Asphalt Concrete Thickness (ft.)	Base Course Thickness (ft.)
Parking areas, other light traffic (T.I. = 5.0)	0.25	0.35
Driveways, truck aisles, other moderate traffic areas (T.I. = 7.0)	0.35	0.35

These recommendations are provided for estimating purposes only. At the completion of rough grading, when the actual soils are more accurately defined, samples should be obtained for additional R-value testing which will serve as a

basis for the actual structural pavement section design. The final testing and design should be completed by the geotechnical engineer of record. All work within parking lot and street areas should be done per the applicable codes, ordinances and requirements of the City of Hemet and under the inspection of that agency.

**Lateral Design:** Resistance to lateral loads will be provided by a combination of friction acting at the base of the slab or foundation and passive earth pressure. A coefficient of friction of 0.45 between soil and concrete may be used with dead load forces only. A passive earth pressure of 400 pounds per square foot, per foot of depth, may be used for the sides of footings poured against recompacted or dense native material. These values may be increased by 33 percent to provide for lateral loads of short duration such as those caused by wind or seismic forces. Passive earth pressure should be ignored within the upper one foot except where confined as beneath a floor slab, for example.

**Trench Wall Stability:** Significant caving did not occur within our exploratory borings. All excavations should be configured per with the requirements of CalOSHA. We would classify the soils as Type C. The classification of the soil and the shoring and/or slope configuration should be the responsibility of the contractor on the basis of the trench depth and the soil encountered. The contractor should have a "competent person" on-site for the purpose of assuring safety within and about all construction excavations.

**Retaining Walls:** Retaining walls may be necessary during construction and/or landscaping. The retaining walls may be designed for an active earth pressure equivalent to that exerted by a fluid weighing not less than 40 pounds per square foot, per foot of depth.

For walls that are restrained, an "At-Rest" lateral earth pressure should be used. This may be taken as an Equivalent Fluid Pressure of 62 pounds per cubic foot with the resultant applied at mid-height.

At least 12 inches of granular material should be used in the backfill behind the walls and water pressure should not be permitted to build up behind retaining walls. The upper 12 to 18 inches of the backfill should consist of soil having a



underground utility lines should be traced out and completely removed from the site. Each end of the abandoned utility line should be securely capped at the entrance and exit to the site to prevent any water from entering the site. Soils loosened due to the removal of trees should be removed and replaced as controlled compacted fill under the direction of the geotechnical engineer.

**2. Preparation of Surfaces to Receive Compacted Fill:** All surfaces to receive compacted fill should be subjected to compaction testing prior to processing. Testing should indicate a relative compaction of at least 85 percent within the unprocessed native soils. If roots or other deleterious materials are encountered or if the relative compaction fails to meet the acceptance criterion, additional overexcavation will be required until satisfactory conditions are encountered. Upon approval, surfaces to receive fill should be scarified, brought to near optimum moisture content, and compacted to a minimum of 90 percent relative compaction.

**3. Placement of Compacted Fill:** Fill materials consisting of on-site soils or approved imported granular soils should be spread in shallow lifts and compacted at near optimum moisture content to a minimum of 90 percent relative compaction.

**4. Preparation of Paving Areas:** During final grading and immediately prior to the placement of concrete or a base course, all surfaces to receive asphalt concrete paving or concrete slabs-on-grade should be processed and compacted to a depth of at least of 12 inches. This may be accomplished by a combination of overexcavation, scarification and recompaction of the surface, and replacement of the excavated material as controlled compacted fill. Compaction of the slab areas should be to a minimum of 90 percent relative compaction. Compaction within the proposed pavement areas should be to a minimum of 95 percent relative compaction for both the subgrade and base course.

**5. Utility Trench Backfill:** It is our opinion that utility trench backfill consisting of the on-site soil types should be placed by mechanical compaction to a minimum of 90 percent relative compaction. This is with

the exception of the upper 12 inches under pavement areas where the minimum relative compaction should be 95 percent. Jetting of the native soils is not recommended.

**6. Testing and Inspection:** During grading tests and observations should be performed by the project geotechnical engineer or his/her representative to verify that the grading is being performed per the project specifications. Field density testing should be performed per the current ASTM D1556 or ASTM D6938 test methods. The minimum acceptable degree of compaction should be 90 percent of the maximum dry density as obtained by the ASTM D1557 test method except where superseded by more stringent requirements, such as beneath pavement or in deep fills. Where testing indicates insufficient density, additional compactive effort should be applied until retesting indicates satisfactory compaction.

Testing should also be conducted to verify that the soils will not subject concrete to sulfate attack and are not corrosive. Testing of any proposed import will be necessary prior to placement on the site. Testing of on-site soils may be done on either a selective or random basis as site conditions indicate.

## **GENERAL**

The findings and recommendations presented in this report are based upon an interpolation of the soil conditions between boring locations. Should conditions be encountered during grading that appears to be different than those indicated by this report, this office should be notified.

We recommend that a pre-job conference be held on the site prior to the initiation of site grading. The purpose of this meeting will be to assure a complete understanding of the recommendations presented in this report as they apply to the actual grading performed.

This report was prepared for Cozad & Fox, Inc. for their use in the design of the Hemet Regional Service Center Parking Lot Improvement Project. This report may only be used by Cozad & Fox, Inc. for this purpose. The use of this report by parties or for other purposes is not authorized without written permission by Inland Foundation Engineering, Inc. Inland Foundation Engineering, Inc. will not be liable for any projects connected with the unauthorized use of this report.

The recommendations of this report are considered to be preliminary. The final design parameters may only be determined or confirmed at the completion of site grading on the basis of observations made during the site grading operation. To this extent, this report is not considered to be complete until the completion of both the design process and the site preparation.

## APPENDIX A

### FIELD EXPLORATION





















For our field exploration, four exploratory borings were excavated by means of a truck mounted rotary auger rig at the approximate locations shown on Figure No. A-7. Logs of the materials encountered were made on the site by a staff geologist. These are presented on Figure Nos. A-3 through A-6.

Representative relatively undisturbed samples were obtained within our borings by driving an 18-inch long thin-walled steel penetration sampler (SPT) with successive 30-inch drops of a 140-pound hammer. The number of blows required to achieve each six inches of penetration were recorded on our boring logs and used for estimating the relative consistency of the subsoils. Two different samplers were used. The first sampler used was a Standard Penetration Test Sampler (SPT) for which published correlations relating the number of hammer blows to the strength of the soil are available. The second sampler type was a Modified California split barrel sampler, which is larger in diameter, carrying brass sample rings having inner diameters of 2.41 inches. Relatively undisturbed samples were removed from the sampler and placed in moisture sealed containers in order to preserve the natural soil moisture content. They were then transported to our laboratory for further observations and testing.

Representative bulk samples were obtained and returned to our laboratory for further testing and observations. The results of this testing are discussed and presented in Appendix B.



## UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D2487)

PRIMARY DIVISIONS		GROUP SYMBOLS		SECONDARY DIVISIONS	
COARSE GRAINED SOILS MORE THAN HALF OF MATERIALS IS LARGER THAN #200 SIEVE SIZE	GRAVELS MORE THAN HALF OF COARSE FRACTION IS LARGER THAN #4 SIEVE	CLEAN GRAVELS (LESS THAN) 5% FINES	GW		WELL GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
		GRAVEL WITH FINES	GP		POORLY GRADED GRAVELS OR GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
		CLEAN SANDS (LESS THAN) 5% FINES	GM		SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES
		SANDS WITH FINES	GC		CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES
	SANDS MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN #4 SIEVE	CLEAN SANDS (LESS THAN) 5% FINES	SW		WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
		SANDS WITH FINES	SP		POORLY GRADED SANDS OR GRAVELLY SANDS, LITTLE OR NO FINES
		SANDS WITH FINES	SM		SILTY SANDS, SAND-SILT MIXTURES
		SANDS WITH FINES	SC		CLAYEY SANDS, SAND-CLAY MIXTURES
FINE GRAINED SOILS MORE THAN HALF OF MATERIALS IS SMALLER THAN #200 SIEVE SIZE	SILTS AND CLAYS LIQUID LIMIT IS LESS THAN 50	ML		INORGANIC SILTS, VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS	
		CL		INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS	
		OL		ORGANIC SILTS AND ORGANIC SILT-CLAYS OF LOW PLASTICITY	
	SILTS AND CLAYS LIQUID LIMIT IS GREATER THAN 50	MH		INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDS OR SILTS, ELASTIC SILTS	
		CH		INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS	
		OH		ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	
	HIGHLY ORGANIC SOILS		PT		PEAT, MUCK AND OTHER HIGHLY ORGANIC SOILS
TYPICAL FORMATIONAL MATERIALS	SANDSTONES		SS		
	SILTSTONES		SH		
	CLAYSTONES		CS		
	LIMESTONES		LS		
	SHALES		SL		

### CONSISTENCY CRITERIA BASES ON FIELD TESTS

RELATIVE DENSITY - COARSE - GRAIN SOIL			CONSISTENCY - FINE-GRAIN SOIL		TORVANE	POCKET ** PENETROMETER	* NUMBER OF BLOWS OF 140 POUND HAMMER FALLING 30 INCHES TO DRIVE A 2 INCH O.D. (1 3/8 INCH I.D.) SPLIT BARREL SAMPLER (ASTM -1586 STANDARD PENETRATION TEST)  ** UNCONFINED COMPRESSIVE STRENGTH IN TONS/SQ.FT. READ FROM POCKET PENETROMETER
RELATIVE DENSITY	SPT * (# BLOWS/FT)	RELATIVE DENSITY (%)	CONSISTENCY	SPT* (# BLOWS/FT)	UNDRAINED SHEAR STRENGTH (tsf)	UNCONFINED COMPRESSIVE STRENGTH (tsf)	
VERY LOOSE	<4	0-15	Very Soft	<2	<0.13	<0.25	
LOOSE	4-10	15-35	Soft	2-4	0.13-0.25	0.25-0.5	
MEDIUM DENSE	10-30	35-65	Medium Stiff	4-8	0.25-0.5	0.5-1.0	
DENSE	30-50	65-85	Stiff	8-15	0.5-1.0	1.0-2.0	
VERY DENSE	>50	85-100	Very Stiff	15-30	1.0-2.0	2.0-4.0	
			Hard	>30	>2.0	>4.0	

#### MOISTURE CONTENT

DESCRIPTION	FIELD TEST
DRY	Absence of moisture, dusty, dry to the touch
MOIST	Damp but no visible water
WET	Visible free water, usually soil is below water table










#### CEMENTATION

DESCRIPTION	FIELD TEST
Weakly	Crumbled or breaks with handling or slight finger pressure
Moderately	Crumbles or breaks with considerable finger pressure
Strongly	Will not crumble or break with finger pressure

### EXPLANATION OF LOGS

# LOG OF BORING B-01

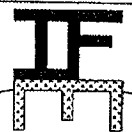
Elevation:	<u>1582.0</u>	Date(s) Drilled:	<u>9/4/14</u>	Logged by:	<u>DL</u>
Drilling Method:	<u>Rotary Auger</u>	Hammer Type:	<u>Auto-Trip</u>	Hammer Weight:	<u>140 lb.</u>
Drilling Rig:	<u>Mobile B-61</u>	Hammer Drop:	<u>30-inches</u>		
Boring Diameter:	<u>8-inches</u>				

DEPTH (ft)	GRAPHIC	USCS	SUMMARY OF SUBSURFACE CONDITIONS			SAMPLES			MOISTURE (%)	DRY UNIT WT. (pcf)	RELATIVE COMPACTION (%)	
			This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered and is representative of interpretations made during drilling. Contrasting data derived from laboratory analysis may not be reflected in these representations.			DRIVE SAMPLE	BULK SAMPLE	SAMPLE TYPE				BLOWS/6"
			<b>ARTIFICIAL FILL.</b> SILTY SAND, fine-grained with gravel, light brown, slightly moist, loose to medium dense, asphalt concrete debris.					BULK				
		SM	<b>SILTY SAND.</b> fine- to medium-grained with trace gravel, light brown, slightly moist, loose to medium dense.					BULK SS	6 6	2	105	84
5								SS	3 4	6	94	76
		SM	<b>SILTY SAND.</b> fine- to medium-grained, gray-brown, slightly moist, medium dense, very weakly cemented.					SS	4 3			
10								SS	4 4			
			End of boring at 10.5 feet. No groundwater or mottling encountered.									

## LOG OF BORING B-02

Elevation: 1582.0      Date(s) Drilled: 9/4/14      Logged by: DL  
 Drilling Method: Rotary Auger      Hammer Type: Auto-Trip  
 Drilling Rig: Mobile B-61      Hammer Weight: 140 lb.  
 Boring Diameter: 8-inches      Hammer Drop: 30-inches

DEPTH (ft)	GRAPHIC	USCS	SUMMARY OF SUBSURFACE CONDITIONS		SAMPLES			BLOWS/6"	MOISTURE (%)	DRY UNIT WT. (pcf)	RELATIVE COMPACTION (%)
			This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered and is representative of interpretations made during drilling. Contrasting data derived from laboratory analysis may not be reflected in these representations.	DRIVE SAMPLE	BULK SAMPLE	SAMPLE TYPE					
	[Cross-hatch pattern]		<b>ARTIFICIAL FILL.</b> SILTY SAND, fine- to medium-grained with trace gravel, light brown, slightly moist, loose to medium dense, concrete fragments.	[Solid black bar]	BULK						
	[Dotted pattern]	SM	<b>SILTY SAND,</b> fine-grained with trace medium, gray-brown, slightly moist, medium dense, massive.	[X-pattern]	SS	7 6	3	103			
5	[Dotted pattern]	SM	<b>SAND with SILT,</b> fine- to coarse-grained, gray-brown, slightly moist, medium dense.	[X-pattern]	SS	5 5	2	104	84		
	[Dotted pattern]	SW SM	<b>SAND with SILT,</b> fine- to coarse-grained, gray-brown, slightly moist, medium dense.	[X-pattern]	SS	5 10					
10	[Dotted pattern]	SM	<b>SILTY SAND,</b> fine-grained with trace medium, brown, moist, medium dense, micaceous.	[X-pattern]	SS	3 4					
			End of boring at 10.5 feet. No groundwater or mottling encountered.								



**INLAND FOUNDATION ENGINEERING, INC.**

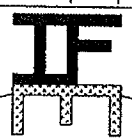
Geotechnical Investigation  
 723 & 749 N. State St.  
 Hemet, CA  
 Project No. C143-050

Figure No.  
  
**A-4**

## LOG OF BORING B-03

Elevation:	1582.0	Date(s) Drilled:	9/4/14	Logged by:	DL
Drilling Method:	Rotary Auger	Hammer Type:	Auto-Trip		
Drilling Rig:	Mobile B-61	Hammer Weight:	140 lb.		
Boring Diameter:	8-inches	Hammer Drop:	30-inches		

DEPTH (ft)	GRAPHIC	USCS	SUMMARY OF SUBSURFACE CONDITIONS		SAMPLES			BLOWS/6"	MOISTURE (%)	DRY UNIT WT. (pcf)	RELATIVE COMPACTION (%)
			This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered and is representative of interpretations made during drilling. Contrasting data derived from laboratory analysis may not be reflected in these representations.	DRIVE SAMPLE	BULK SAMPLE	SAMPLE TYPE					
			<b>ARTIFICIAL FILL</b> , fine- to medium-grained with trace gravel, light-brown, slightly moist, loose.								
5		SM	<b>SILTY SAND</b> , fine- to medium-grained, gray-brown, slightly moist, medium dense.			SS	4 5	2	104	84	
						SS	4 6	2	102	82	
						SS	7 9				
10		SM	<b>SILTY SAND</b> , fine-grained with trace medium, gray-brown, moist, medium dense, weakly cemented.			SS	5 6				
			End of boring at 10.5 feet. No groundwater or mottling encountered.								






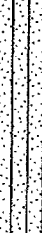



**INLAND FOUNDATION ENGINEERING, INC.**

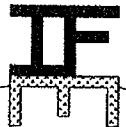
Geotechnical Investigation  
 723 & 749 N. State St.  
 Hemet, CA  
 Project No. C143-050

Figure No.  
  
**A-5**

# LOG OF BORING B-04

Elevation:	1582.0	Date(s) Drilled:	9/4/14	Logged by:	DL
Drilling Method:	Rotary Auger	Hammer Type:	Auto-Trip		
Drilling Rig:	Mobile B-61	Hammer Weight:	140 lb.		
Boring Diameter:	8-inches	Hammer Drop:	30-inches		

DEPTH (ft)	GRAPHIC	USCS	SUMMARY OF SUBSURFACE CONDITIONS			SAMPLES			BLOWS/6"	MOISTURE (%)	DRY UNIT WT. (pcf)	RELATIVE COMPACTION (%)
			This summary applies only at the location of the boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered and is representative of interpretations made during drilling. Contrasting data derived from laboratory analysis may not be reflected in these representations.	DRIVE SAMPLE	BULK SAMPLE	SAMPLE TYPE						
			<b>ARTIFICIAL FILL, SILTY SAND</b> , fine- to medium-grained with trace gravel, brown, dry to slightly moist, loose.			BULK						
		SM	<b>SILTY SAND</b> , fine-grained with trace medium, gray-brown, slightly moist, medium dense.			BULK SS		4 4	3	107	86	
5		SM	<b>SILTY SAND</b> , fine- to medium-grained, gray-brown, moist, medium dense, massive.			SS		5 9	5	108	87	
10		SM	<b>SILTY SAND</b> , fine-grained, light brown, moist, medium dense, weakly cemented, friable. Interbedded with occasional lenses of silt.			SS		3 4				
			End of boring at 10.5 feet. No groundwater or mottling encountered.									



**INLAND FOUNDATION ENGINEERING, INC.**

Geotechnical Investigation  
 723 & 749 N. State St.  
 Hemet, CA  
 Project No. C143-050

Figure No.  
  
**A-6**

## APPENDIX B

### LABORATORY TESTING

Representative bulk soil samples were obtained in the field and returned to our laboratory for additional observations and testing. Laboratory testing was generally performed in two phases. The first phase consisted of testing in order to evaluate the apparent compaction of the existing natural soil and the general engineering classifications of the soils across the site. This testing was performed in order to estimate the engineering characteristics of the soil and to serve as a basis for selecting samples for the second phase of testing. The second phase consisted of soil mechanics and analytical testing. This testing included direct shear testing, R-value testing, and testing to estimate the concentration of water-soluble sulfate, pH, resistivity and chlorides. These tests were performed in order to provide a means of developing specific design recommendations based on the strength and corrosive characteristics of the soil.

### CLASSIFICATION AND COMPACTION TESTING

**Unit Weight and Moisture Content:** Each ring sample was weighed and measured to evaluate its unit weight. A small portion of each sample was then subjected to testing to evaluate its moisture content. This testing was performed per the current ASTM Standards D2937 and D2216. This was used in order to evaluate the dry density of the soil in its natural condition. The results of this testing are shown on the Boring Logs (Figure Nos. A-3 through A-6).

**Maximum Density-Optimum Moisture:** Representative soil types were selected for maximum density tests. This testing was performed per the current ASTM Standard D1557 test method A. The results of this testing are presented graphically on Figure No. B-3. The maximum densities are compared to the field densities of the soil to evaluate the existing relative compaction to the soil. This is shown on the boring logs, and is useful in estimating the strength and compressibility of the soil.

**Classification Testing:** Two soil samples were selected for classification testing. This testing consists of mechanical grain size analyses and sand equivalent tests. This testing was performed per the current ASTM Standards D422. The results of this testing are very useful in detecting variations in the soils and in selecting samples for further testing. The results of this testing are presented on Figure No. B-4.

## SOIL MECHANICS TESTING

**R-value Testing:** One sample was selected for R-value testing. This test measures the ability of soil to resist lateral deformation under applied vertical loads, and is used in developing parameters for pavement structural sections. Testing was performed in accordance with Caltrans Test Method 301. The results of this testing are shown on Figure No. B-5.

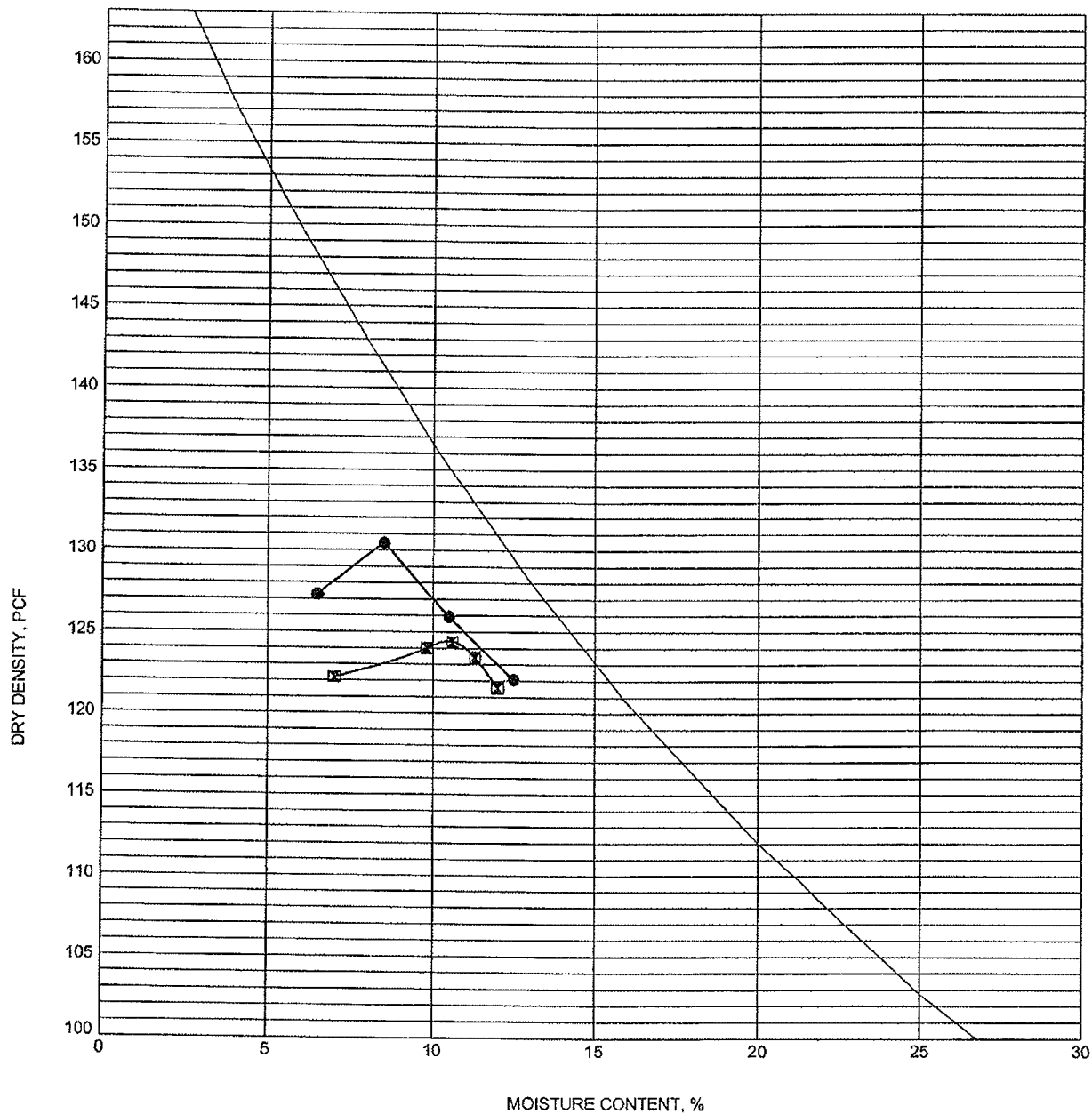
## ANALYTICAL TESTING

One sample was selected to determine the concentration of soluble sulfates, chlorides, pH level, and resistivity of and within the on-site soils. The following table presents the results of this testing:

Sample Location	Sample Depth (ft.)	Water-Soluble Sulfates (%)	Chlorides (ppm)	Minimum Resistivity (ohm-cm)	pH
B-01	1.3-6.0	<0.001	135	13,788	7.8

## GENERAL

All laboratory testing has been conducted in conformance with the applicable ASTM test methods by personnel trained and supervised in conformance with our QA/QC policy. Our test data only relates to the specific soils tested. Soil conditions typically vary and any significant variations should be reported to our laboratory for review and possible testing. The data presented in this report are for the use of Cozad & Fox, Inc. only and may not be reproduced or used by others without written approval of Inland Foundation Engineering, Inc.



Specimen Identification	Classification	Max. Density	MC%
● B-01 0.0	SILTY SAND SM	130.5	9.0
☐ B-01 1.3	SILTY SAND SM	124.5	10.5

PROJECT Geotechnical Investigation  
723 & 749 N. State St.

PROJECT NO. C143-050  
 DATE \_\_\_\_\_

**MAXIMUM DENSITY-OPTIMUM MOISTURE CURVES**

Inland Foundation Engineering, Inc

FIGURE NO. B-3





### R-VALUE DETERMINATION

Project: Cozad and Fox  
 Project No: C143-050

Date: 9/4/2014

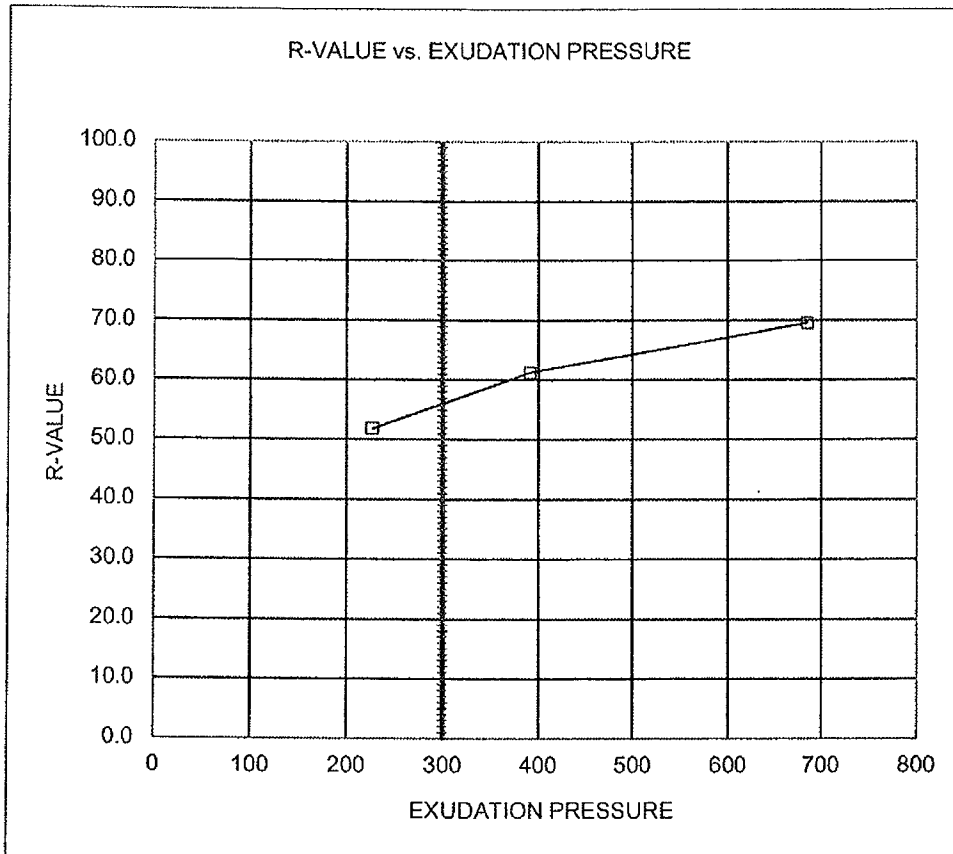
Sample No.: B-01

Sample Location: County of Riverside Hemet Regional Service Center Parking Lot

Sample Depth: 1.3-6.0

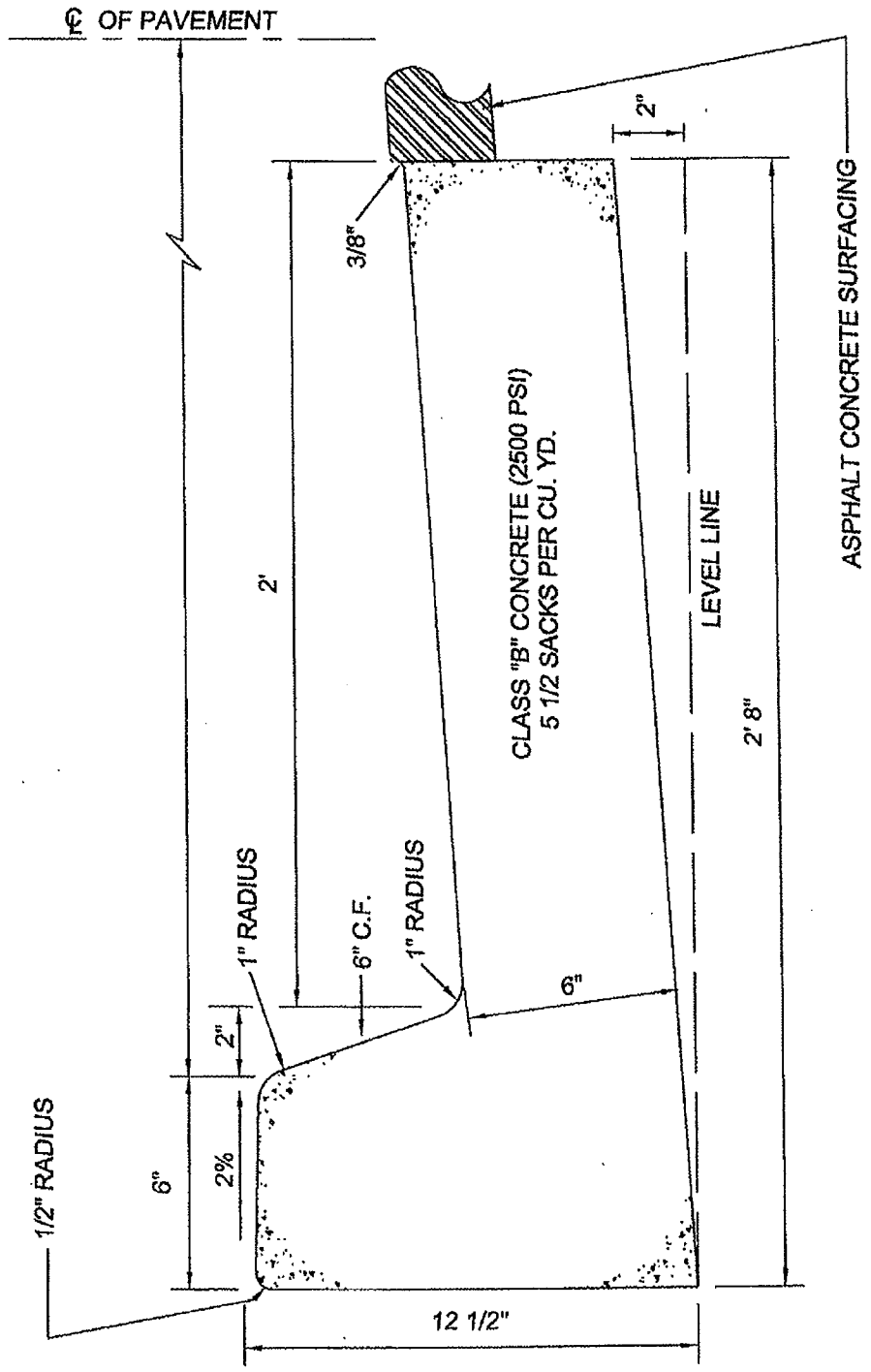
Test Specimen	A	B	C	D
Sample Weight(gm)	1100	1100	1100	
Initial Moisture	4.6	4.6	4.6	
Moisture Added(ml)	55	60	65	
Compaction Moist.%	9.8	10.3	10.8	
Wet Wght. (grms)	1155	1160	1165	
Height (in)	2.6	2.59	2.63	
Correction	1	1	1	
Density (pcf)	122.6	123.0	121.2	
Stabilometer@2000	32	40	51	
Displacement	4.36	4.74	4.96	
R-Value	69.6	61.3	51.9	
Exudation Press.	686	392	227	
Exp. Initial	640	607	633	
Exp. Final	648	612	633	
Exp. Pressure	34.64	21.65	0	
Exp. Thickness	0.27	0.17	0.00	

R-VALUE by Exudation: 56



**APPENDIX B**

Standard Drawings



NOTE: ADD A 3" SHINER AT THE FLOWLINE

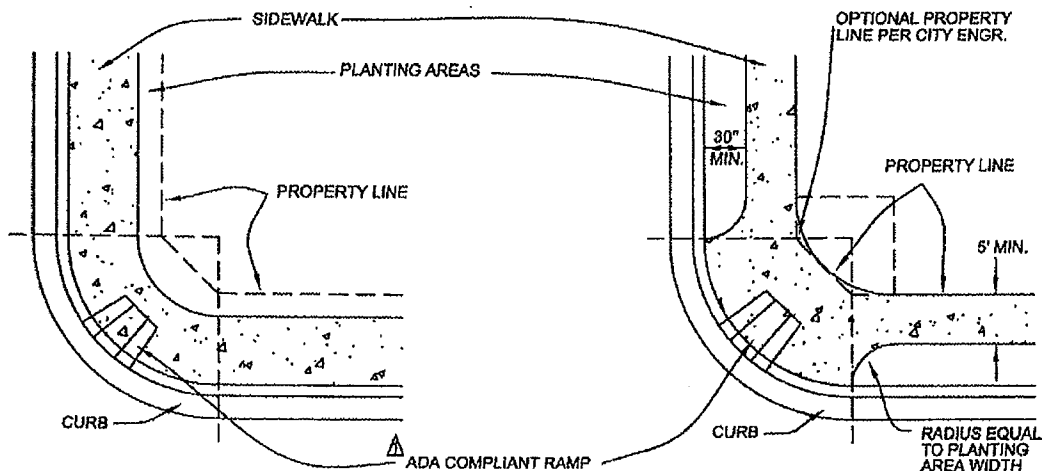
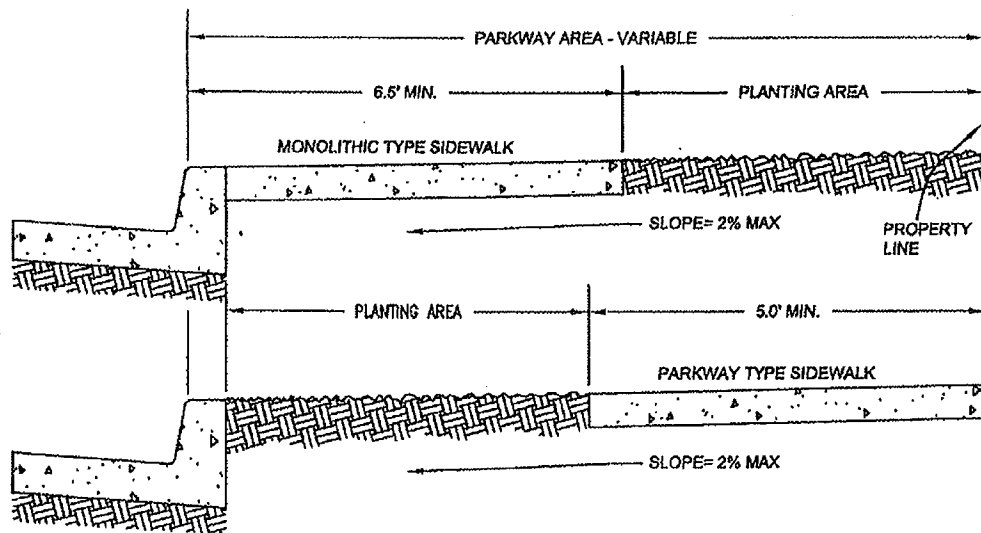


**City of Hemet**  
**PUBLIC WORKS DEPARTMENT**  
**ENGINEERING DIVISION**  
 510 E. FLORIDA AVENUE  
 HEMET, CA 92543  
 (951) 765-2360

**TYPE "A" CURB**

<i>[Signature]</i>	MAR 2009
MICHAEL A. GOW, PW DIRECTOR/CITY ENG. R.C.E. 54164	DATE
REVISIONS	
N:\P\W\100\COH_STDS\REVISIONS\STREETS\C-200.dwg	
N.Beltran 2009	

1  
 OF 1 SHEETS  
**STANDARD NO.**  
**C-200**



**NOTES**

1. 2% (MAX.) SLOPE FOR DRAINAGE FROM CURB TO PROPERTY LINE.
2. SIDE WALK THICKNESS TO BE NOT LESS THAN 4".
3. VARIABLE DISTANCES SHOWN ON STANDARD TYPICAL STREET SECTIONS.
4. SIDE WALKS SHALL BE CLEAR OF ALL OBSTRUCTIONS SUCH AS POWER POLES, LIGHT STANDARDS, ECT. ALL GAS VAULTS, WATER METER BOXES, ECT. SHALL BE ADJUSTED TO FINISH GRADE.
5. CONCRETE 5½ SACK MIX PER CUBIC YARD. (2500 PSI)
6. SPACING AND POSITIONING OF TREE WELLS TO BE DETERMINED BY DEPT. OF PUBLIC WORKS.
7. SCORE MARK EVERY 5', SHEAR-PLANE EVERY 20', EXPANSION JOINT EVERY 60'.

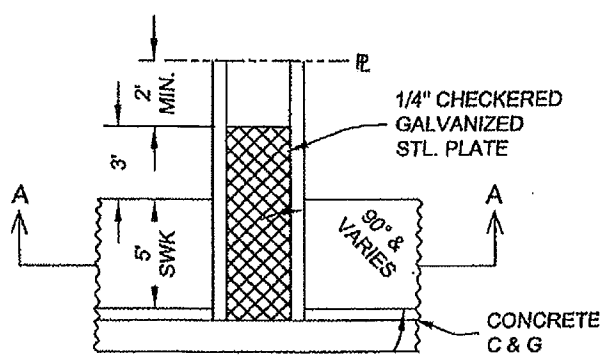
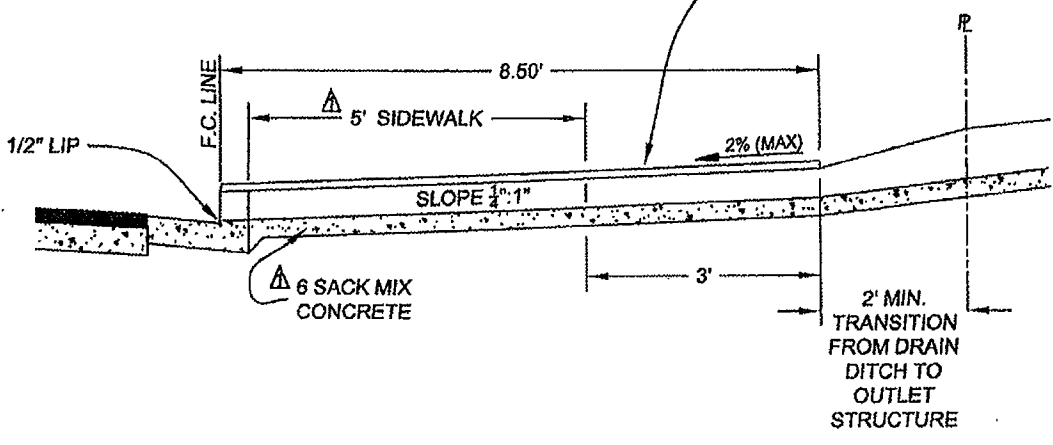
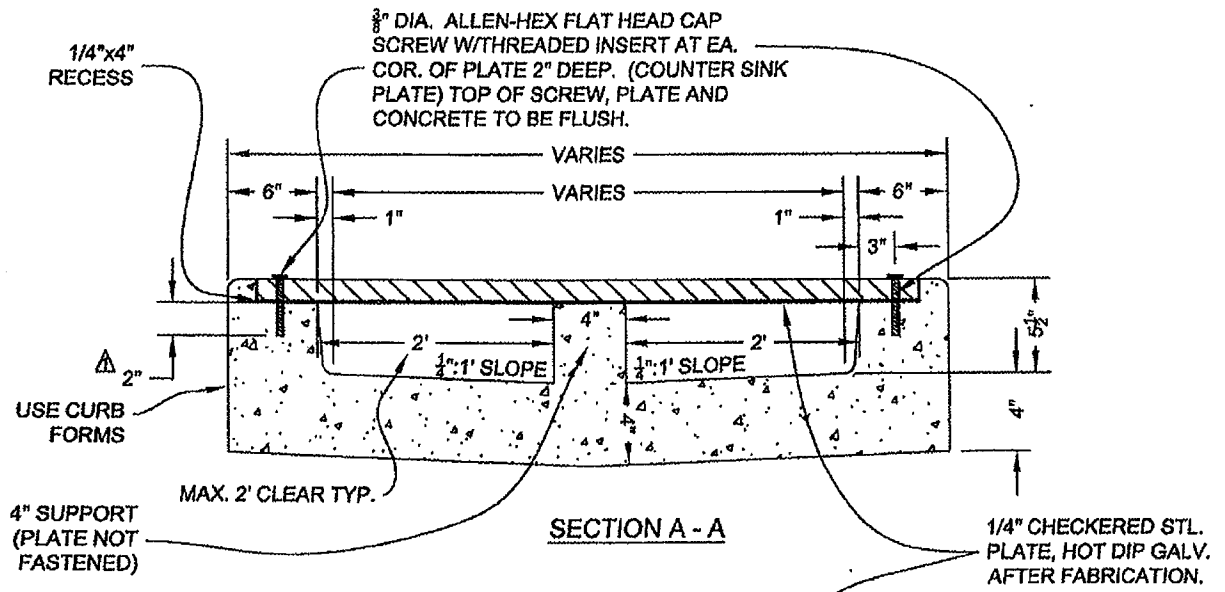


**City of Hemet**  
**PUBLIC WORKS DEPARTMENT**  
**ENGINEERING DIVISION**  
 510 E. FLORIDA AVENUE  
 HEMET, CA 92543  
 (951) 765-2360

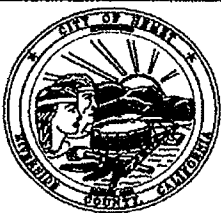
**SIDEWALK AND PARKWAY**

		MAR 2011
JORGE L. BIAGIONI, PRINCIPAL ENGINEER R.C.E. 33751		DATE
REVISIONS		
A	MAR 2011	REVISED ADA RAMP CALL OUT
N:\P\W\4100\COH_STDS\REVISIONS\CONCRETE\C-215.dwg		N.Beltran 2011

**1**  
 OF 1 SHEETS  
**STANDARD NO.**  
**C-215**



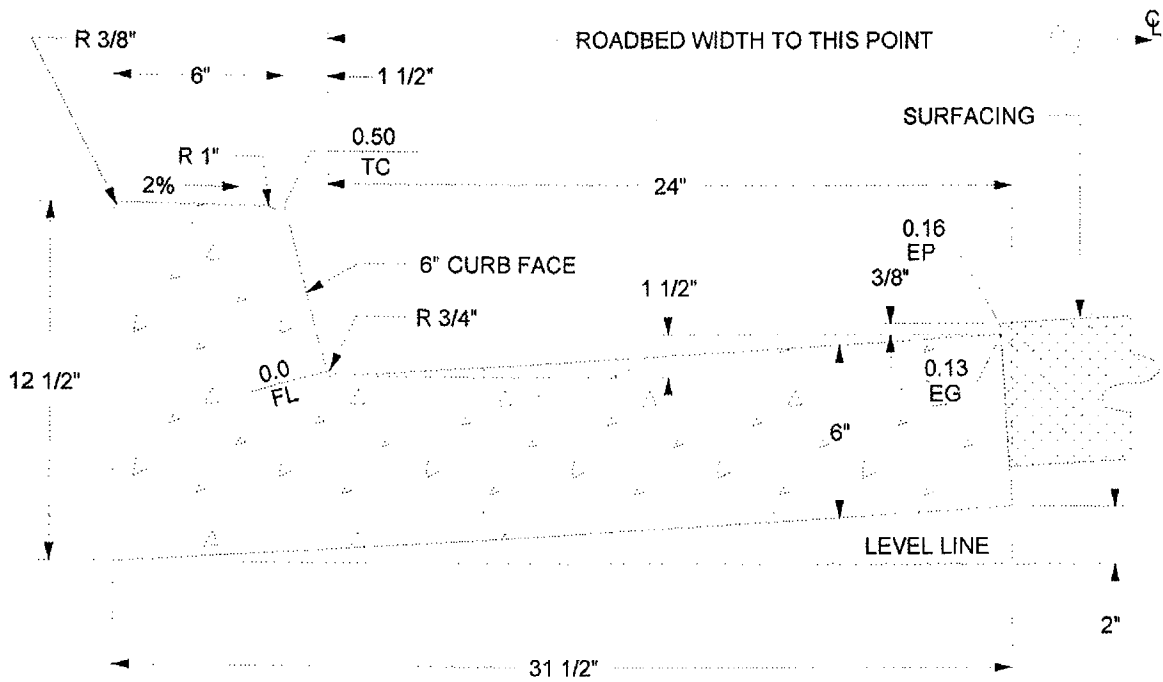
**DRAINAGE OUTLET**



**City of Hemet**  
**PUBLIC WORKS DEPARTMENT**  
**ENGINEERING DIVISION**  
 510 E. FLORIDA AVENUE  
 HEMET, CA 92543  
 (951) 765-2350

**CURB OUTLET  
(WITH STEEL PLATE)**

<i>JLB</i>	MAR 2011	1
JORGE L. BIAGIONI, PRINCIPAL ENGINEER R.C.E. 33751	DATE	OF 1 SHEETS
REVISIONS		
A MARCH 2011 REVISIONS TO DIMENSIONS		
N:\PW\4100\COH_STDS\REVISIONS\DRAINAGE\D-306.dwg	N.Beltran 2011	<b>STANDARD NO. D-306</b>



CLASS "B" CONCRETE  
 1.601 CU. FT. / L.F.  
 1 CU. YD. = 16.86 L.F.

ABBREVIATIONS:  
 TC = TOP OF CURB  
 FL = FLOWLINE  
 EG = EDGE OF GUTTER  
 EP = EDGE OF PAVEMENT

APPROVED BY:

*George A. Johnson*  
 DIRECTOR OF TRANSPORTATION  
 GEORGE A. JOHNSON, RCE 42328

DATE: 05/01/07



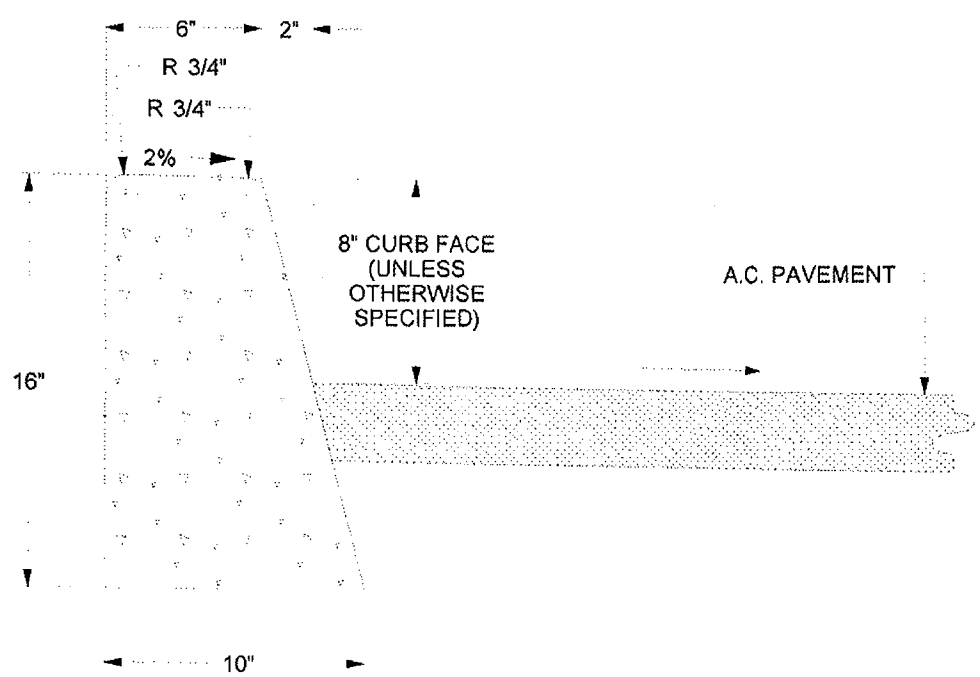
COUNTY OF RIVERSIDE

**TYPE A-6 CURB**

REVISIONS	REV.	BY:	APR'D	DATE	REV.	BY:	APR'D	DATE
8-71, 9-88	1				4			
2-90, 11-04	2				5			
	3				6			

STANDARD NO. 200

← ROADBED WIDTH TO THIS POINT →



CLASS "B" CONCRETE  
 0.888 CU FT. / L.F.  
 1 CU. YD. = 30.41 L.F.

APPROVED BY:  
*George A. Johnson*  
 DIRECTOR OF TRANSPORTATION  
 GEORGE A. JOHNSON, RCE 42328

DATE: 05/01/07



COUNTY OF RIVERSIDE

**TYPE "D" CURB**

REVISIONS	REV. BY:	APR'D	DATE	REV. BY:	APR'D	DATE
8-71, 2-90	1			4		
11-04	2			5		
	3			6		

STANDARD NO. 204



FOR BILLING INQUIRIES:  
CALL (951) 368-9710  
EMAIL [billinginquiry@pe.com](mailto:billinginquiry@pe.com)

DATE	ORDER NUMBER	PONumber	PRODUCT	SIZE	AMOUNT
11/7/15	0010108382		PE Riverside	2 x 190 Li	551.00
11/14/15	0010108382		PE Riverside	2 x 190 Li	494.00

Invoice text: Re-Bid for Hemet Regional Service Center

RECEIVED RIVERSIDE COUNTY  
 CLERK / BOARD OF SUPERVISORS  
 2015 NOV 19 AM 10:45

*EDA  
3-3 of 11/03/15*

**Legal Advertising Invoice**

**BALANCE DUE**

1,045.00

SALES/CONTACT INFORMATION		ADVERTISER INFORMATION		
	BILLING DATE	BILLED ACCOUNT NUMBER	ADVERTISER/CLIENT NUMBER	ADVERTISER/CLIENT NAME
Maria Tinajero 951-368-9225	11/14/2015	1100141323	1100141323	BOARD OF SUPERVISORS

PLEASE DETACH AND RETURN THIS PORTION WITH YOUR REMITTANCE

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NEWS GROUP  
**THE PRESS-ENTERPRISE**

Legal Advertising Invoice

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BOARD OF SUPERVISORS		
BILLING DATE	BILLED ACCOUNT NUMBER	ADVERTISER/CLIENT NUMBER
11/14/2015	1100141323	1100141323
BALANCE DUE	ORDER NUMBER	TERMS OF PAYMENT
1,045.00	0010108382	DUE UPON RECEIPT

BILLING ACCOUNT NAME AND ADDRESS

REMITTANCE ADDRESS

BOARD OF SUPERVISORS  
COUNTY OF RIVERSIDE  
'P.O. BOX 1147'  
RIVERSIDE, CA 92502

The Press-Enterprise  
File 1555  
1801 W Olympic Blvd  
Pasadena, CA 91199-1555

# THE PRESS-ENTERPRISE

1825 Chicago Ave, Suite 100  
Riverside, CA 92507  
951-684-1200  
951-368-9018 FAX

## PROOF OF PUBLICATION (2010, 2015.5 C.C.P)

Publication(s): The Press-Enterprise

PROOF OF PUBLICATION OF

Ad Desc.: Re-Bid for Hemet Regional Service Center

I am a citizen of the United States. I am over the age of eighteen years and not a party to or interested in the above entitled matter. I am an authorized representative of THE PRESS-ENTERPRISE, a newspaper in general circulation, printed and published daily in the County of Riverside, and which newspaper has been adjudicated a newspaper of general circulation by the Superior Court of the County of Riverside, State of California, under date of April 25, 1952, Case Number 54446, under date of March 29, 1957, Case Number 65673, under date of August 25, 1995, Case Number 267864, and under date of September 16, 2013, Case Number RIC 1309013; that the notice, of which the annexed is a printed copy, has been published in said newspaper in accordance with the instructions of the person(s) requesting publication, and not in any supplement thereof on the following dates, to wit:

11/07, 11/14/2015

I certify (or declare) under penalty of perjury that the foregoing is true and correct.

Date: Nov 14, 2015  
At: Riverside, California



BOARD OF SUPERVISORS  
COUNTY OF RIVERSIDE  
P.O. BOX 1147  
RIVERSIDE, CA 92502

Ad Number: 0010108382-01

P.O. Number:

Ad Copy:

### NOTICE INVITING BIDS

**NOTICE IS HEREBY GIVEN** that the County of Riverside ("County") Invites sealed Bids for the construction of the following project ("Work"):

#### HEMET REGIONAL SERVICE CENTER PARKING LOT IMPROVEMENTS

Bids shall be prepared in conformance with the Instructions to Bidders and other Bidding Documents. Bids must be received, by hand delivery or mail, by the Clerk of the Board located on the 1st floor of the County Administrative Center, 4080 Lemon Street, Riverside, CA 92501, no later than the Bid Closing Deadline of 10:00 a.m. on 12/07/2015, to be thereafter on said date and at said location publicly opened and read aloud. The Bidder assumes sole responsibility for timely receipt of its Bid.

On and after **11/09/2015**, and up to amount of hours (72) hours prior to the Bid Closing Deadline, copies of Bidding Documents will be available to Bidders for pick-up by Bidder at, or for mailing to Bidder upon written request by Bidder submitted to Mission Reprographics, 2050 E. La Cadena Dr., Suite L, Riverside, California, 92507, upon payment. Notify Gary Schwalbe, Mission Reprographics at (951) 686-8828 ahead of time for plan production and cost information; website: <http://www.missionreprographics.com>. The Bidding Documents may also be viewed in person between the hours of **9:00 a.m. and 4:00 p.m.**, Monday through Friday, (except Holidays) at: Riverside County Economic Development Agency, 3403 Tenth Street, Suite 400, Riverside, CA 92501 For further information, contact Susana Orozco at the County of Riverside Economic Development Agency, whose telephone number is (951) 955-9127.

Pursuant to Labor Code section 1771.1, any contractor bidding, or subcontractor to be listed on a bid proposal subject to Public Contract Code section 4104, shall not be qualified to bid after March 1, 2015, unless currently registered and qualified to perform public works pursuant to Labor Code section 1725.5. No Contractor or subcontractor may enter into a contract (after April 1, 2015) without proof of current registration to perform public works.

The Bidder receiving the Award by the County is required:

- (1) to furnish a Performance Bond and Payment Bond as provided in the Instructions to Bidders and other Bidding Documents;
- (2) both at the time Bidder submits its Bid and other Bid Submittals and at the time of Award, to:  
(a) hold a contracting license, active and in good standing, issued by the Contractors State License Board for the State of California for the following license classification: **Class "A" license in the State of California or provide a combination of current and active specialty licenses as appropriate and qualified to perform the Work specified in the Contract Documents;**
- (3) to comply with the provisions of the California Labor Code, including, without limitation, Sections 1771.4, 1773.1, 1774, 1775 and 1776 of the California Labor Code and including, without limitation, the obligations to pay the general prevailing rates of wages in the locality in which the Work is to be performed and comply with Section 1777.5 of the California Labor Code governing employment of apprentices. Copies of the prevailing rates of per diem wages are on file at California State Department of Industrial Relations, 464 West Fourth St., Suite 348, San Bernardino, CA 92401, and are available to any interested party on request.

**THIS IS A PUBLIC WORKS PROJECT AND SUBJECT TO COMPLIANCE MONITORING AND ENFORCEMENT BY THE DEPARTMENT OF INDUSTRIAL RELATIONS. The awarded prime contractor shall post job site notices as prescribed by regulation starting January 1, 2015. Contractor or subcontractor shall furnish records specified in Labor Code section 1776 to the Labor Commissioner. Substitution of securities for any moneys withheld by County shall be permitted as provided for by Section 22300 of the California Public Contract Code.**

Federal Requirements: This project is being financed with Community Development Block Grant funds (24 CFR Part 570) from the U.S. Department of Housing and Urban Development (HUD). All contracts pertaining to this project will be subject to certain requirements including but not limited to: HUD Federal Labor Standards Provisions (HUD 4010) - Davis-Bacon Prevailing Wages; Section 3 Economic Opportunities Requirements (24 CFR Part 135); Special Federal Provisions; Additional Federal Requirements; and Executive Order #11246. Information pertaining to the Federal requirements is incorporated into this bid document and is on file with the County of Riverside Economic Development Agency.

Prevailing Wages: Pursuant to the California Labor Code, the governing board of the Owner has obtained from the director of the Department of Industrial Relations determination of general prevailing rates of per diem wages applicable to the work, and for holiday and overtime work, including employer payments for health and welfare, pension, vacation, and similar purposes, as set forth on the schedule which is on file at the principal office of the Owner, and which will be made available to any interested person upon request. The Contractor shall comply with all applicable provisions of the California State Labor Code prevailing wages and Compliance of State of Cali-

California Department of Industrial Relations division of Apprenticeship Standards Labor. These wages are set forth in the General Prevailing Wage Rates for this project, available from the California Department of Industrial Relations' Internet web site at [www.dir.ca.gov](http://www.dir.ca.gov). Future effective prevailing wage rates which have been predetermined, and are on file with the California Department of Industrial Relations, are referenced but not printed in the general prevailing wage rates.

The Federal minimum wage requirements, as predetermined by the Secretary of Labor, are set forth in these documents issued for bidding purposes, referenced to herein as the "Special Federal Provisions", and in copies of these documents which may be examined at the office described above where the project plans, special provisions, and proposal forms may be seen. Addenda to modify the minimum wage rates, if necessary, will be issued to holders of the Project Bid Documents.

For any specific labor classification employed on this project, the higher of the State Prevailing Wage or Federal Prevailing Wage (Davis-Bacon) must be paid.

**Mandatory Job Walk and Pre-Bid Conference: Bidders are required to attend a pre-bid meeting and mandatory job walk which will be conducted by the County at 10:00 a.m., on November 18, 2015. The meeting will be held at the Hemet Regional Service Center, located at 749 N. State Street, Hemet, CA 92543. The purpose of the meeting is to acquaint bidders with the site conditions, mandatory bidding requirements, and Special Federal Provisions including HUD 4010 Labor Compliance and Section 3 Equal Economic Opportunities requirements. Further, prospective subcontractors will be notified of subcontracting and material supply opportunities.**

Each bid proposal must be accompanied a certified or cashier's check, or bid bond issued by a surety admitted and regulated by the State of California and further, if the work or project is financed in whole or in part with federal grant or loan funds, listed in the Treasury Department's most current Circular 570 (bid bond shall be submitted on the form included in the Contract Documents or on an equivalent form approved by the County) for an amount not less than ten percent (10%) of the maximum amount bid. Said check or bond shall be made payable to the COUNTY OF RIVERSIDE, and when delivered with a proposal, shall constitute a guarantee that the bidder will, if award is made in accordance with the terms or said bidder's proposal, execute a Contract in the County's standard form, together with Labor Code Certification thereon; furnish Contract Performance and Payment Bonds with a corporate surety or sureties satisfactory to the County, or equivalent substitution in lieu of bonds, each for not less than one-hundred percent (100%) of the bid price and furnish Certificates of Insurance evidencing that all insurance coverage required by the contract has been secured. Capitalized terms used herein shall have the meanings assigned to them in the Bidding Documents. For information contact: Economic Development Agency, 3403 10th St., Riverside, CA 92501.

Dated: November 4, 2015

KECIA HARPER-IHEM, Clerk of the Board of Supervisors  
By: Cecilia Gil, Board Assistant 11/7, 11/14