

backing system

ecologix^e

features and benefits

Performance precoat, fiberglass reinforced thermoplastic layer laminated with a fiber matrix secondary containing 88% post consumer material

backing process

6-step process

1. High performance precoat for maximum tuft bind and moisture resistance.

2. Thermoplastic laminate for superior delamination strength.

3. Fiberglass reinforcement for unmatched stability.

4. Thermoplastic laminate.

5. Attached post consumer recycled fiber matrix pad for comfort

under foot.

6. Die-cut into carpet tile.

primary

Synthetic

stabilizer

Fiberglass Mat

weight

85 oz/sy

secondary thickness

.150 inch

dimensions

24" x 24" (standard); optional sizes available

pad density

8.5 - 9.5 lbs. per cubic foot

installation

Full Spread Shaw Sureset N5000 Pressure Sensitive releasable

adhesive

Apply with a 3/8 foam paint roller

Eco*Logix ES: Shaw's environmental self-adhesive system

(custom option)

warranty

Lifetime Commercial

shawcontractgroup.com

To download details on installation, specifications, and

warranties on-line

shaw inforum

call 1.877.502.7429 for personal assistance

APPENDIX C SUMMARY DATA SHEET

BUILDING DATA								
Building Name:	UNITED STATES D	ISTRICT CO	URT				05/05/0045	
Building Address:		RIVERSIDE, C	A 92501		 	Date:	05/25/2015	
Latitude:	33.9762047		Longitude: -11	7.3752267			· · · · · · · · · · · · · · · · · · ·	
Year Built:	2000	Year(s	Remodeled:		0::1	By:		
Area (sf):	62,487		Length (ft): 268	3'	Original I	Design Code:	1994 UBC 97'	
No. of Stories:	3+BASEMENT	_	Story Height:14	l'		Width (ft):	70'	
USE Industri	ial Office Wa	rehouse 🗌	Hospital Resi	donei I		Total Height:		
CONSTRUCTION D			- respitati - Resi		Educational 🔽	Other: COL	KINOUSE	
Gravity Load	Structural System: REIN	NFORCED CO	ONCRETE MASO	NRY				
			NCRETE MASONR				GLASS	
Exterior L			NCRETE MASONR			Openings?	GLASS	
Roof N			STEEL FRAMING	·		Openings?	GLASS	
Intermediat			CONCRETE/STEEL	FRAMING				
	Ground Floor: CON	CRETE SLAB	ON GRADE					
	Columns: STE					Para Lui	COMODETE O	45.044.05
General Cond	ition of Structure: _ EXC	ELLENT				Foundation:	CONCRETE SL	AB ON GRADE
	els Below Grade? ONE					<u> </u>		
Special Feature	s and Comments: NON	NE						
LATERAL-FORCE-R	ESISTING SYSTEM							
	- TOTEM		The State of					
	0		Longitudinal			Trans	verse	
	System:	ORDINARY S	TEEL CONCENTRICA	LLY BRACED F	RAME ORDINAR	Y STEEL CONC	ENTRICALLY BRA	CED FRAME
	Vertical Elements:		STEEL		•	STEEL		
	Diaphragms:	CON	CRETE IN STEEL PAN	<u> </u>		CONCRETE IN	STEEL PAN	
TVALUATION TO	Connections:		BOLTED/WELDED		*****	BOLTEDA		
EVALUATION DATA								
BSE-	IN Spectral Response Accelerations:	C	1.00					
	Soil Factors:	$S_{D_x} =$	D		$S_{D1} =$	0.619		
BSE-	-1E Spectral Response	Class =			F _u =	1.00	$F_v =$	1.50
	Accelerations:	$S_{xx} =$	N/A		C -	N/A		
	Level of Seismicity:		12	Pe	$S_{xi} =$ rformance Level:	N		
	Building Period:	<i>T</i> =	0.269 SECS		romance Level:			
	Spectral Acceleration:	S,, =	N/A					
	Modification Factor:	$C_mC_1C_2 =$	1.1	Buildir	ig Weight: W =	NOT DOCUM	MENTED	
	Pseudo Lateral Force:	<i>V</i> =	,		g meight. W =	NOT BOOM	ALIVIED .	
		$C_m C_1 C_2 S_a W =$	NOT DOCUMENT	ED				
BUILDING CLASSIFIC	CATION:							
REQUIRED TIER 1 CI	HECKLISTS		V					
Basic Configuration			Yes [☑	No				
	_ Structural Checklis	st	\ <u>\</u>	=				
Nonstructural Com	ponent Checklist		⊠ ⊠					
	ON REQUIREMENT:	N/A	וארו	Ц				
					<u></u>			

Project: 3470 12TH STREET DISTRICT COURTHOUSE	Location: 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: NEVILLE PEREIRA, P.E.	Date:5/25/2015

TIER 1 CHECKLISTS

16.1 BASIC CHECKLIST

Very Low Seismicity

Structural Components

C NC N/A U

LOAD PATH: The structure shall contain a complete, well-defined load path, including structural elements and connections, that serves to transfer the inertial forces associated with the mass of all elements of the building to the foundation. (Commentary: Sec. A.2.1.1. Tier 2: Sec. 5.4.1.1)

C NC N/A

WALL ANCHORAGE: Exterior concrete or masonry walls that are dependent on the diaphragm for lateral support are anchored for out-of-plane forces at each diaphragm level with steel anchors, reinforcing dowels, or straps that are developed into the diaphragm. Connections shall have adequate strength to resist the connection force calculated in the Quick Check procedure of Section 4.5.3.7. (Commentary: Sec. A.5.1.1. Tier 2: Sec. 5.7.1.1)

Project: 3470 12TH	H STREET US DISTRICT COURTHOUSE Location:	3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: NE	EVILLE PEREIRA, P.E. Date: 05/2	5/2015
40.4.04.0		
	SAFETY BASIC CONFIGURATION CHECKLIST	
Low Seismicity Building System		
General		
C NC N/A U	LOAD PATH: The structure shall contain a complete, we connections, that serves to transfer the inertial forces ass to the foundation. (Commentary: Sec. A.2.1.1. Tier 2: Se	ociated with the mass of all elements of the building
C NC N/A U		ne building being evaluated and any adjacent building
C NO N/A	MEZZANINES: Interior mezzanine levels are braced inc to the seismic-force-resisting elements of the main struct	ependently from the main structure or are anchored
Building Configura	ation	, , , , , , , , , , , , , , , , , , , ,
C NO N/A J	WEAK STORY: The sum of the shear strengths of the sedirection is not less than 80% of the strength in the adjact Sec. 5.4.2.1)	ismic-force-resisting system in any story in each ent story above. (Commentary: Sec. A2.2.2. Tier 2:
C NO N/A J	SOFT STORY: The stiffness of the seismic-force-resistin seismic-force-resisting system stiffness in an adjacent sto force-resisting system stiffness of the three stories above.	ry above or less than 80% of the average seismic.
C NO N/A	VERTICAL IRREGULARITIES: All vertical elements in the foundation. (Commentary: Sec. A.2.2.4. Tier 2: Sec. 2.2.4.)	the seismic-force-resisting system are continuous to
C NC N/A U		dimension of the seismic-force-resisting system of
C NC N/A U	MASS: There is no change in effective mass more than 5 penthouses, and mezzanines need not be considered. (Constitution of the considered	0% from one story to the next. Light roofs, nmentary: Sec. A.2.2.6. Tier 2: Sec. 5.4.2.5)
C IC N/A U	TORSION: The estimated distance between the story cen than 20% of the building width in either plan dimension.	ter of mass and the story center of rigidity is less
	ity: Complete the Following Items in Addition to the Ite	ns for Low Seismicity.
Geologic Site Haza	ards	
C N N/A U	LIQUEFACTION: Liquefaction-susceptible, saturated, lo seismic performance shall not exist in the foundation soil (Commentary: Sec. A.6.1.1. Tier 2: 5.4.3.1)	ose granular soils that could jeopardize the building's at depths within 50 ft under the building.

High Seismicity: Complete the Following Items in Addition to the Items for Low and Moderate Seismicity.

failure. (Commentary: Sec. A.6.1.2. Tier 2: 5.4.3.1)

anticipated. (Commentary: Sec. A.6.1.3. Tier 2: 5.4.3.1)

Foundation Configuration

N/A

N/A

C NONA U OVERTURNING: The ratio of the least horizontal dimension of the seismic-force-resisting system at the foundation level to the building height (base/height) is greater than 0.6S_a. (Commentary: Sec. A.6.2.1. Tier 2: Sec. 5.4.3.3)

TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C. (Commentary: Sec. A.6.2.2. Tier 2: Sec. 5.4.3.4)

SLOPE FAILURE: The building site is sufficiently remote from potential earthquake-induced slope failures or rockfalls to be unaffected by such failures or is capable of accommodating any predicted movements without

SURFACE FAULT RUPTURE: Surface fault rupture and surface displacement at the building site are not

Project:3470 12TH ST	REET BANKRUPCY COURTHOUSE	Location: 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: NEVIL	LE PEREIRA, P.E.	Date: 5/25/2015
16.1.2IO IMMEDIA	TE OCCUPANCY BASIC CONTINUE OF	
Very Low Seismicity	TE OCCUPANCY BASIC CONFIGURAT	TION CHECKLIST
Building System		
General		
	0.47	
b	OAD PATH: The structure shall contain a cond connections, that serves to transfer the ineulding to the foundation. (Commentary: Sec.	mplete, well-defined load path, including structural elements rtial forces associated with the mass of all elements of the A.2.1.1. Tier 2: Sec. 5.4.1.1)
C IC N/A U A	DJACENT BUILDINGS: The clear distance greater than 4% of the height of the shorter uilding types: W1, W1a, and W2. (Commentations)	between the building being evaluated and any adjacent building building. This statement need not apply for the following ary: Sec. A.2.1.2. Tier 2: Sec. 5.4.1.2)
C NO N/A D M	IEZZANINES: Interior mezzanine levels are	braced independently from the main structure or are anchored nain structure. (Commentary: Sec. A.2.1.3. Tier 2: Sec. 5.4.1.3)
Building Configuration		,
	EAK STORY: The sum of the shear strength rection shall not be less than 80% of the streier 2: Sec. 5.4.2.1)	is of the seismic-force-resisting system in any story in each ngth in the adjacent story above. (Commentary: Sec. A.2.2.2.
fo	rce-resisting system stiffnessof the three stori	ce-resisting system in any story shall not be less than 70% of an adjacent story above or less than 80% of the average seismicies above. (Commentary: Sec. A.2.2.3. Tier 2: Sec. 5.4.2.2)
C Nd N/A J VI	ERTICAL IRREGULARITIES: All vertical e e foundation. (Commentary: Sec. A.2.2.4. Tie	elements in the seismic-force-resisting system are continuous to er 2: Sec. 5.4.2.3)
(0	EOMETRY: There are no changes in the net ore than 30% in a story relative to adjacent stommentary: Sec. A.2.2.5. Tier 2: Sec. 5.4.2.4	horizontal dimension of the seismic-force-resisting system of tories, excluding one-story penthouses and mezzanines.
	and mezzanines need not be consid	ore than 50% from one story to the next. Light roofs, lered. (Commentary: Sec. A.2.2.6. Tier 2: Sec. 5.4.2.5)
tha	ORSION: The estimated distance between the an 20% of the building width in either plan d	story center of mass and the story center of rigidity is less imension. (Commentary: Sec. A.2.2.7. Tier 2: Sec. 5.4.2.6)
Low Seismicity: Comp	lete the Following Items in Additionto the	Items for Very Low Seismicity.
Geologic Site Hazards		•
	QUEFACTION: Liquefaction-susceptible, sat smic performance shall not exist in the found ommentary: Sec. A.6.1.1. Tier 2: 5.4.3.1)	turated, loose granular soils that could jeopardize the building's lation soils at depths within 50 ft under the building.
	OPE FAILURE: The building site is sufficiently such failures or is lure. (Commentary: Sec. A.6.1.2. Tier 2: 5.4.)	ntly remote from potential earthquake-induced slope failures or capable of accommodating any predicted movements without 3.1)
C NO N/A J SU		offlire and surface displacement at the building of
	ismicity: Complete the Following Items in	
Foundation Configurat	ion	
C NO N/A J OV	ERTURNING: The ratio of the least horizontal	dimension of the seismic-force-resisting system at the foundation

level to the building height (base/height) is greater than 0.6S_a. (Commentary: Sec. A.6.2.1. Tier 2: Sec. 5.4.3.3) TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces

(Commentary: Sec. A.6.2.2. Tier 2: Sec. 5.4.3.4)

where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C.

N/A

Project Title: Engineer: Project Descr:

US DISTRICT COURT EVALUATION N. PEREIRA Project ID: SEISMIC EVALUATION

ASCE Seismic Base Shear

CE	CRAI	0	ALA	1 VSIS
`\F	11/11/11		MM	1 Y < 1 <

Lic. #: KW-06008839

Risk Category				Ca	culations per ASCE 7-10
Risk Category of Building or Other Structure : "H" :	All Buildings and other	er structures	except thos	e listed as Category I. III, and IV	ASCE 7-10, Page 2, Table 1-5-1
Seismic Importance Factor =	1			,	ASCE 7-10. Page 5, Table 1.5-
Gridded Ss & S1values ASCE-7-10 Standard				•	ASCE 7-10 11.4.1
Max. Ground Motions, 5% Damping :		Latitude	=	33.991 deg North	
S _S = 1.5 g, 0.2 sec response		Longitude	=	117.371 deg West	
$S_1 = 0.6185 \text{ g. } 1.0 \text{ sec response}$		Location :	Riverside,		
Site Class, Site Coeff. and Design Category					
Site Classification "D": Shear Wave Velocity 600 to 1,200 ft/s	sec	=	D		ASCE 7-10 Table 20.3-
Site Coefficients Fa & Fv	Fa	=	1.00	AS	CE 7-10 Table 11.4-1 & 11.4-2
(using straight-line interpolation from table values)	Fv	=	1.50		
Maximum Considered Earthquake Acceleration	S _{MS} = Fa * Ss	=	1.500		ASCE 7-10 Eq. 11.4-1
	S M1 = Fv * S1	=	0.928		ASCE 7-10 Eq. 11.4-2
Design Spectral Acceleration	S _{DS} = S * 2/3	=	1.000		ASCE 7-10 Eq. 11.4-3
	$S_{D1} = S_{M1}^{*} 2/3$	=	0.619		ASCE 7-10 Eq. 11.4-4
Seismic Design Category	DI WII	=	D		ASCE 7-10 Table 11.6-1 & -2
Resisting System					ASCE 7-10 Table 12.2-1
Response Modification Coefficient "R" = 3.0 System Overstrength Factor "Wo" = 3.0 Deflection Amplification Factor "Cd" = 2.5 NOTE! See ASCE 7-10 for all applicable footnotes.	00 Cate 60 Cate Cate Cate	g height Lir gory "A & B gory "C" Lin gory "D" Lin gory "E" Lin gory "F" Lin	" Limit: nit: nit: nit:	No Limit Limit = 160 Not Permitted Not Permitted Not Permitted	
Redundancy Factor					ASCE 7-10 Section 12.3.4
Seismic Design Category of D, E, or F therefore Redunda	ancy Factor " p " = 1.3	3			
Lateral Force Procedure					ASCE 7-10 Section 12.8.2
	orce Procedure" is	being used	d according	to the provisions of ASCE 7-10 12.8	
Determine Building Period					Use ASCE 12.8-7
Structure Type for Building Period Calculation: All Other S					
"x " value = 0.75	" : Height from base	to highest le	evel =	46.0 ft	
"Ta "Approximate fundemental period using Eq. 12.8-7; "TL": Long-period transition period per ASCE 7-10 Maps 2		Ta = Ct • (hi	n^x) =	0.353 sec 8.000 sec	
" Cs " Response Coefficient	Bu	ilding Perio	d " Ta " Calc	ulated from Approximate Method selected	= 0.353 sec ASCE 7-10 Section 12.8.1.1
S _{DS} : Short Period Design Spectral Response	= 1.000)	From Fa	12.8-2, Preliminary Cs	0.000
2 PC 2000 Lenor Design 2 Decrai Response		,	r rom Eq.	12.0 E, I Iominiary Co	= 0.333
			From Fa	12.8-3 & 12.8-4 Cs need not exceed	
" R " : Response Modification Factor "I" : Seismic Importance Factor	= 3.00)		12.8-3 & 12.8-4 , Cs need not exceed 12.8-5 & 12.8-6, Cs not be less than	= 0.584 = 0.103

Lic. #: KW-06008839

Project Title: Engineer: Project Descr:

US DISTRICT COURT EVALUATION
N. PEREIRA Project ID:
SEISMIC EVALUATION

ASCE Seismic Base Shear

Printed. 1 JUN 2015. 11:09AM

File = c:\Users\Neville\DROPBO-T\UOBRECRD\2015\15283R-1\LEMONA-1\347012-1.EC6

ENERCALC, INC. 1983-2015, Build:6.15.1.19, Ver:6.15.1.19

Licensee: Neville Pereira, P.E.

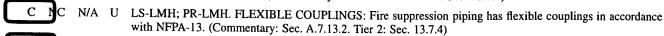
Cs =	0.3333 from 12.8.1.1		W(s	ee Sur	n Wi below) =	0.00 k	CE 7-10 Section 12.8.1
			•		V = Cs * W =	•	
/ertical Distribut	ion of Seismic Forces			,,,,,,,,	. 03 11 -	0.00 k	
k ": hx exponent bas	sed on Ta = 1.00					AS	CE 7-10 Section 12.8.3
ble of building Weigh							
Level #	Wi : Weight	Hi : Height	(Wi * Hi) ^k	Cvx	Fx=Cvx * V	Sum Story Shear	Sum Story Momer
Sum Wi ≃	0.00 k	Sum Wi * Hi =	0.00 k-ft		Total Base Shear =	0.00 k	
					, 512, 5400 5170di	Base Moment ≈	0.0 k-ft
Diaphragm Force	s : Seismic Design Catego	ory "B" to "F"					ASCE 7-10 12.10.1.1
Level #	Wi	Fi	Sum Fi		Sum Wi	Env	7302 7 10 12.10.1.1
Wpx	Weight a	level of diaphragm	and other structure e			Брх	_
Fi	Design L	ateral Force annlied	at the level	emen	is attached to it.		
Sum Fi	Sum of "I	at. Force" of curren	at tile level. t level blus all levels :	ahovo			
MIN Reg'd Force	@ Level 0.20 * S	* I * Wnx	ricvei pius ali ieveis a	DOVE			
	e@Level 0.40 * S _[
i pr. Design ruit	ce @ Level Wpx * St	IM(x->n) Fi / SUM(x->n) wi, $x = Current$	level,	n = Top Level		

Project: 3420 & 3470 12TH STREET COURTHOUSES	Location: 3420 & 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: NEVILLE PEREIRA, P.E.	Date: _5/25/2015

16.17 NONSTRUCTURAL CHECKLIST

Life Safety Systems

-	C N	c	N/A	U	LS-LMH; PR-LMH. FIRE SUPPRESSION PIPING: Fire suppression piping is anchored and braced in accordance with NFPA-13. (Commentary: Sec. A.7.13.1, Tier 2: Sec. 13.7.4)
					accordance with NFPA-13. (Commentary: Sec. A.7.13.1. Tier 2: Sec. 13.7.4)



C NC N/A U LS-LMH; PR-LMH. EMERGENCY POWER: Equipment used to power or control life safety systems is anchored or braced. (Commentary: Sec. A.7.12.1. Tier 2: Sec. 13.7.7)

C NC NA U LS-LMH; PR-LMH. STAIR AND SMOKE DUCTS: Stair pressurization and smoke control ducts are braced and have flexible connections at seismic joints. (Commentary: Sec. A.7.14.1. Tier 2: Sec. 13.7.6)

C NC NA U 15-MH; PR-MH. SPRINKLER CEILING CLEARANCE: Penetrations through panelized ceilings for fire suppression devices provide clearances in accordance with NFPA-13. (Commentary: Sec. A.7.13.3. Tier 2: Sec. 13.7.4)

C N/A U LS-not required; PR-LMH. EMERGENCY LIGHTING: Emergency and egress lighting equipment is anchored or braced. (Commentary: Sec. A.7.3.1. Tier 2: Sec. 13.7.9)

Hazardous Materials

C NONA U LS-LMH; PR-LMH. HAZARDOUS MATERIAL EQUIPMENT: Equipment mounted on vibration isolators and containing hazardous material is equipped with restraints or snubbers. (Commentary: Sec. A.7.12.2. Tier 2: 13.7.1)

LS-LMH; PR-LMH. HAZARDOUS MATERIAL STORAGE: Breakable containers that hold hazardous material, including gas cylinders, are restrained by latched doors, shelf lips, wires, or other methods. (Commentary: Sec. A.7.15.1. Tier 2: Sec. 13.8.4)

LS-MH; PR-MH. HAZARDOUS MATERIAL DISTRIBUTION: Piping or ductwork conveying hazardous materials is braced or otherwise protected from damage that would allow hazardous material release. (Commentary: Sec. A.7.13.4. Tier 2: Sec. 13.7.3 and 13.7.5)

NE N/A U LS-MH; PR-MH. SHUT-OFF VALVES: Piping containing hazardous material, including natural gas, has shut-off valves or other devices to limit spills or leaks. (Commentary: Sec. A.7.13.3. Tier 2: Sec. 13.7.3 and 13.7.5)

NO N/A U LS-LMH; PR-LMH. FLEXIBLE COUPLINGS: Hazardous material ductwork and piping, including natural gas piping, has flexible couplings. (Commentary: Sec. A.7.15.4, Tier 2: Sec.13.7.3 and 13.7.5)

LS-MH; PR-MH. PIPING OR DUCTS CROSSING SEISMIC JOINTS: Piping or ductwork carrying hazardous material that either crosses seismic joints or isolation planes or is connected to independent structures has couplings or other details to accommodate the relative seismic displacements. (Commentary: Sec. A.7.13.6. Tier 2: Sec.13.7.3, 13.7.5, and 13.7.6)

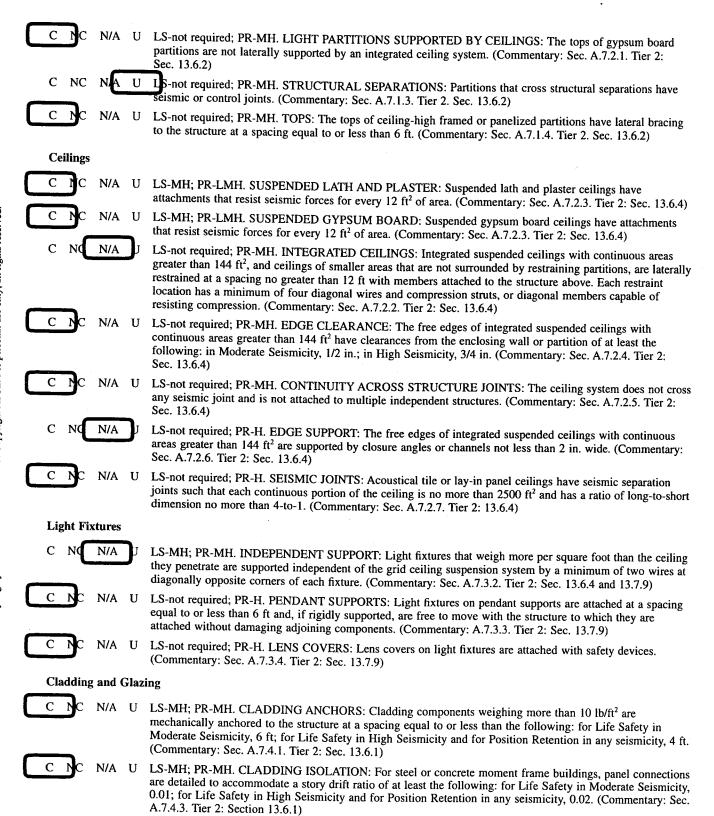
Partitions

N/A

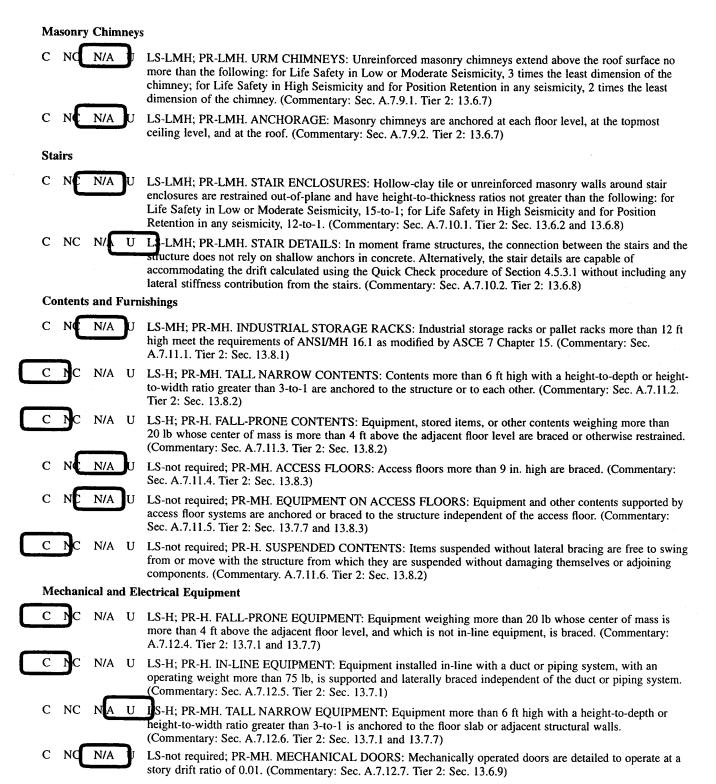
C NO N/A J LS-LMH; PR-LMH. UNREINFORCED MASONRY: Unreinforced masonry or hollow-clay tile partitions are braced at a spacing of at most 10 ft in Low or Moderate Seismicity, or at most 6 ft in High Seismicity. (Commentary: Sec. A.7.1.1. Tier 2: Sec. 13.6.2)

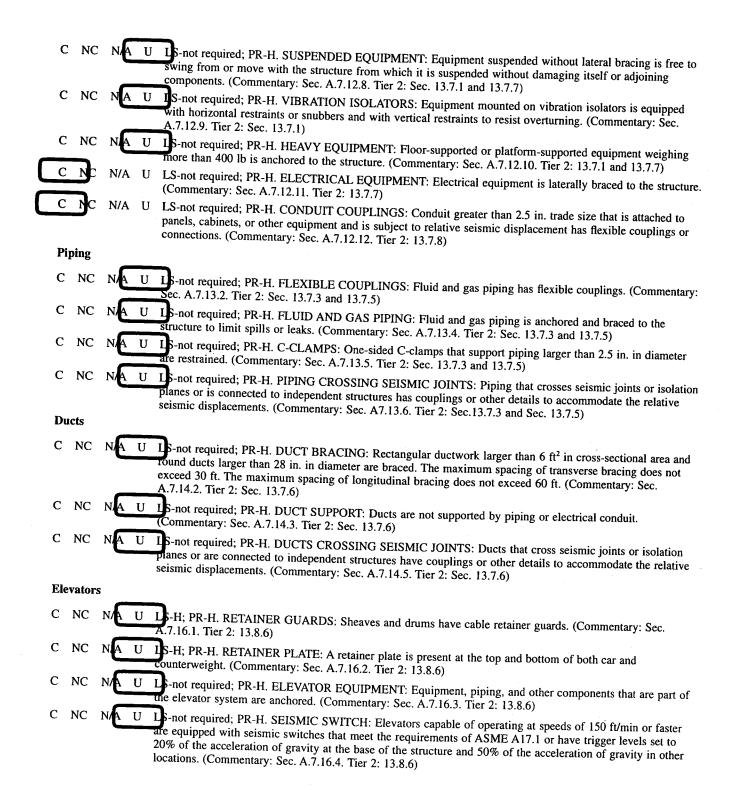
N/A U LS-LMH; PR-LMH. HEAVY PARTITIONS SUPPORTED BY CEILINGS: The tops of masonry or hollow-clay tile partitions are not laterally supported by an integrated ceiling system. (Commentary: Sec. A.7.2.1. Tier 2: Sec. 13.6.2)

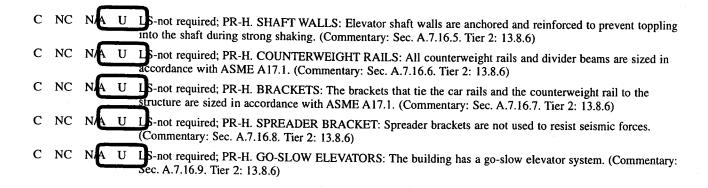
C NO N/A J LS-MH; PR-MH. DRIFT: Rigid cementitious partitions are detailed to accommodate the following drift ratios: in steel moment frame, concrete moment frame, and wood frame buildings, 0.02; in other buildings, 0.005. (Commentary A.7.1.2 Tier 2: Sec. 13.6.2)



С	I C	N/A	U	LS-MH; PR-MH. MULTI-STORY PANELS: For multi-story panels attached at more than one floor level, panel connections are detailed to accommodate a story drift ratio of at least the following: for Life Safety in Moderate Seismicity, 0.01; for Life Safety in High Seismicity and for Position Retention in any seismicty, 0.02. (Commentary: Sec. A.7.4.4. Tier 2: Sec. 13.6.1)
С] C	N/A	U	LS-MH; PR-MH. PANEL CONNECTIONS: Cladding panels are anchored out-of-plane with a minimum number of connections for each wall panel, as follows: for Life Safety in Moderate Seismicity, 2 connections; for Life Safety in High Seismicity and for Position Retention in any seismicity, 4 connections. (Commentary: Sec. A.7.4.5. Tier 2: Sec. 13.6.1.4)
С	NC	N/A	U	LS-MH; PR-MH. BEARING CONNECTIONS: Where bearing connections are used, there is a minimum of two bearing connections for each cladding panel. (Commentary: Sec. A.7.4.6. Tier 2: Sec. 13.6.1.4)
С	I)C	N/A	U	LS-MH; PR-MH. INSERTS: Where concrete cladding components use inserts, the inserts have positive anchorage or are anchored to reinforcing steel. (Commentary: Sec. A.7.4.7. Tier 2: Sec. 13.6.1.4)
С	NC	N/A) u	LS-MH; PR-MH. OVERHEAD GLAZING: Glazing panes of any size in curtain walls and individual interior or exterior panes over 16 ft ² in area are laminated annealed or laminated heat-strengthened glass and are detailed to remain in the frame when cracked. (Commentary: Sec. A.7.4.8: Tier 2: Sec. 13.6.1.5)
Ma	asonry	Vene	er	
С	NO	N/A]	LS-LMH; PR-LMH. TIES: Masonry veneer is connected to the backup with corrosion-resistant ties. There is a minimum of one tie for every 2-2/3 ft ² , and the ties have spacing no greater than the following: for Life Safety in Low or Moderate Seismicity, 36 in.; for Life Safety in High Seismicity and for Position Retention in any seismicity, 24 in. (Commentary: Sec. A.7.5.1. Tier 2: Sec. 13.6.1.2)
С	NO	N/A) 1	LS-LMH; PR-LMH. SHELF ANGLES: Masonry veneer is supported by shelf angles or other elements at each floor above the ground floor. (Commentary: Sec. A.7.5.2. Tier 2: Sec. 13.6.1.2)
Ç	NQ	N/A)	LS-LMH; PR-LMH. WEAKENED PLANES: Masonry veneer is anchored to the backup adjacent to weakened planes, such as at the locations of flashing. (Commentary: Sec. A.7.5.3. Tier 2: Sec. 13.6.1.2)
С	N	N/A)	LS-LMH; PR-LMH. UNREINFORCED MASONRY BACKUP: There is no unreinforced masonry backup. (Commentary: Sec. A.7.7.2. Tier 2: Section 13.6.1.1 and 13.6.1.2)
С	Nd	N/A	ال ح	LS-MH; PR-MH. STUD TRACKS: For veneer with metal stud backup, stud tracks are fastened to the structure at a spacing equal to or less than 24 in. on center. (Commentary: Sec. A.7.6.1. Tier 2: Section 13.6.1.1 and 13.6.1.2)
С	Nd	N/A	ت ار	LS-MH; PR-MH. ANCHORAGE: For veneer with concrete block or masonry backup, the backup is positively anchored to the structure at a horizontal spacing equal to or less than 4 ft along the floors and roof. (Commentary: Sec. A.7.7.1. Tier 2: Section 13.6.1.1 and 13.6.1.2)
C	NO	N/A		LS-not required; PR-MH. WEEP HOLES: In veneer anchored to stud walls, the veneer has functioning weep holes and base flashing. (Commentary: Sec. A.7.5.6. Tier 2: Section 13.6.1.2)
С	NG	N/A)	LS-not required; PR-MH. OPENINGS: For veneer with metal stud backup, steel studs frame window and door openings. (Commentary: Sec. A.7.6.2. Tier 2: Sec. 13.6.1.1 and 13.6.1.2)
Pa	rapets	s, Cori	nices	, Ornamentation, and Appendages
С	NC	N/A	U	LS-LMH; PR-LMH. URM PARAPETS OR CORNICES: Laterally unsupported unreinforced masonry parapets or cornices have height-to-thickness ratios no greater than the following: for Life Safety in Low or Moderate Seismicity, 2.5; for Life Safety in High Seismicity and for Position Retention in any seismicity, 1.5. (Commentary: Sec. A.7.8.1. Tier 2: Sec. 13.6.5)
С	_ .j c 	N/A	U	LS-LMH; PR-LMH. CANOPIES: Canopies at building exits are anchored to the structure at a spacing no greater than the following: for Life Safety in Low or Moderate Seismicity, 10 ft; for Life Safety in High Seismicity and for Position Retention in any seismicity, 6 ft. (Commentary: Sec. A.7.8.2. Tier 2: Sec. 13.6.6)
С	NE	N/A) U	LS-MH; PR-LMH. CONCRETE PARAPETS: Concrete parapets with height-to-thickness ratios greater than 2.5 have vertical reinforcement. (Commentary: Sec. A.7.8.3. Tier 2: Sec. 13.6.5)
С	DN	N/A	J	LS-MH; PR-LMH. APPENDAGES: Cornices, parapets, signs, and other ornamentation or appendages that extend above the highest point of anchorage to the structure or cantilever from components are reinforced and anchored to the structural system at a spacing equal to or less than 6 ft. This checklist item does not apply to parapets or cornices covered by other checklist items. (Commentary: Sec. A.7.8.4. Tier 2: Sec. 13.6.6)







CERTIFICATE OF SEISMIC COMPLIANCE EXISTING BUILDING

Date	5/29	/201	5

NEVILLE PER		served REET, RI	as engi VERSIDE CA	neer in charg	e of the	seismic
re following cha	racteristics:					•
r\v.z	No. of Stories	4		Арргох, Агеа:	62,487	
ode 1994 UBC	Year of Design	Code:	1997	Year of Constr	ruction:	2000
his bu≅ding at t Seismic Safety vodology:	he Life Safety F for Existing F	erform	ance Lev y Owned	el as set forth and Leased	in the IC Building	SSC RP Is, using
วก						
on .						
on						
explain below)						
f this evaluation	n must be atta	ched to	this Ce	tificate		
building chara	cteristics and to	the ext	ent nermi	tted by this levi	el of eva the Life	fuation it e Safety
building chara I subject Build	cteristics and to	the ext	ent nermi	tted by this levi	the Life	e Safety
	uilding located the following char the RM2 The RM3 The	per RM2 No. of Stories No. of Stories No. of Stories Pear of Design The Seismic Safety for Existing Foodlogy: One	per RM2 No. of Stories: 4 Description of Storie	the following characteristics: Per RM2 No. of Stories: 1994 UBC Year of Design Code: 1997 This building at the Life Safety Performance Levis Seismic Safety for Existing Federally Owned and on on	the following characteristics: No. of Stories: 4 Approx. Area: Per RM2 Year of Design Code: 1997 Year of Constructions building at the Life Safety Performance Level as set forth Seismic Safety for Existing Federally Owned and Leased modology:	the following characteristics: Approx. Area: 62,487 Mo. of Stories: 4 Approx. Area: 62,487 Approx. Area: 62,487 Approx. Area: 62,487 Approx. Area: 62,487 Mode 1994 UBC Year of Design Code: 1997 Approx. Area: 62,487 Year of Construction: 1997 This building at the Life Safety Performance Level as set forth in the IC Seismic Safety for Existing Federally Owned and Leased Building and Codology: On on

Comments:

Attach ASCE/SEI 31 Checklist(s) Structural, Nonstructural, and Geologic Site Hazards and Foundation:

APPENDIX C SUMMARY DATA SHEET

15
···
2
·
E SLAB ON GRADE
Y BRACED FRAME
- DidioED Ioune
F _v =
_
· · · · · · · · · · · · · · · · · · ·

Project: 3470 12TH STREET DISTRICT COURTHOUSE	Location: 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by:NEVILLE PEREIRA, P.E.	Date:

TIER 1 CHECKLISTS

16.1 BASIC CHECKLIST

Very Low Seismicity

Structural Components

C NC N/A

N/A U LOAD PATH: The structure shall contain a complete, well-defined load path, including structural elements and connections, that serves to transfer the inertial forces associated with the mass of all elements of the building to the foundation. (Commentary: Sec. A.2.1.1. Tier 2: Sec. 5.4.1.1)

C NC N/A

WALL ANCHORAGE: Exterior concrete or masonry walls that are dependent on the diaphragm for lateral support are anchored for out-of-plane forces at each diaphragm level with steel anchors, reinforcing dowels, or straps that are developed into the diaphragm. Connections shall have adequate strength to resist the connection force calculated in the Quick Check procedure of Section 4.5.3.7. (Commentary: Sec. A.5.1.1. Tier 2: Sec. 5.7.1.1)

Project:	Location: 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: NEVILLE PEREIRA, P.E.	Date:

		ismicity		AFETY BASIC CONFIGURATION CHECKLIST
В	uildin	g Systen	n	
G	enera	!		
С		N/A	U	LOAD PATH: The structure shall contain a complete, well defined load path, including structural elements at connections, that serves to transfer the inertial forces associated with the mass of all elements of the building to the foundation. (Commentary: Sec. A.2.1.1. Tier 2: Sec. 5.4.1.1)
С	` لـــ	N/A	U	ADJACENT BUILDINGS: The clear distance between the building being evaluated and any adjacent building is greater than 4% of the height of the shorter building. This statement shall not apply for the following building types: W1, W1a, and W2. (Commentary: Sec. A.2.1.2. Tier 2: Sec. 5.4.1.2)
С		N/A	ţ	MEZZANINES: Interior mezzanine levels are braced independently from the main structure or are anchored to the seismic-force-resisting elements of the main structure. (Commentary: Sec. A.2.1.3. Tier 2: Sec. 5.4.1.3
В	uildin	g Configi	urati	ion
C	Nd	N/A]	WEAK STORY: The sum of the shear strengths of the seismic-force-resisting system in any story in each direction is not less than 80% of the strength in the adjacent story above. (Commentary: Sec. A2.2.2. Tier 2: Sec. 5.4.2.1)
С	Nd	N/A] ı	SOFT STORY: The stiffness of the seismic-force-resisting system in any story is not less than 70% of the seismic-force-resisting system stiffness in an adjacent story above or less than 80% of the average seismic-force-resisting system stiffness of the three stories above. (Commentary: Sec. A.2.2.3. Tier 2: Sec. 5.4.2.2)
С	Nd	N/A)	VERTICAL IRREGULARITIES: All vertical elements in the seismic-force-resisting system are continuous the foundation. (Commentary: Sec. A.2.2.4. Tier 2: Sec. 5.4.2.3)
С		N/A	U	GEOMETRY: There are no changes in the net horizontal dimension of the seismic-force-resisting system of more than 30% in a story relative to adjacent stories, excluding one-story penthouses and mezzanines. (Commentary: Sec. A.2.2.5. Tier 2: Sec. 5.4.2.4)
С		N/A	U	MASS: There is no change in effective mass more than 50% from one story to the next. Light roofs, penthouses, and mezzanines need not be considered. (Commentary: Sec. A.2.2.6. Tier 2: Sec. 5.4.2.5)
С	i jo	N/A	U	TORSION: The estimated distance between the story center of mass and the story center of rigidity is less than 20% of the building width in either plan dimension. (Commentary: Sec. A.2.2.7. Tier 2: Sec. 5.4.2.6)

Geologic Site Hazards

C NC N/A U LIQUEFACTION: Liquefaction-susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance shall not exist in the foundation soils at depths within 50 ft under the building. (Commentary: Sec. A.6.1.1. Tier 2: 5.4.3.1)

C No N/A U SLOPE FAILURE: The building site is sufficiently remote from potential earthquake-induced slope failures or rockfalls to be unaffected by such failures or is capable of accommodating any predicted movements without failure. (Commentary: Sec. A.6.1.2. Tier 2: 5.4.3.1)

C NC N/A U SURFACE FAULT RUPTURE: Surface fault rupture and surface displacement at the building site are not anticipated. (Commentary: Sec. A.6.1.3. Tier 2: 5.4.3.1)

High Seismicity: Complete the Following Items in Addition to the Items for Low and Moderate Seismicity.

Foundation Configuration

C NO N/A U OVERTURNING: The ratio of the least horizontal dimension of the seismic-force-resisting system at the foundation level to the building height (base/height) is greater than 0.6S_a. (Commentary: Sec. A.6.2.1. Tier 2: Sec. 5.4.3.3)

C NO N/A J TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces

TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C. (Commentary: Sec. A.6.2.2. Tier 2: Sec. 5.4.3.4)

Proje	ect: _	347	0 12TH	STREET BANKRUPCY COURTHOUSE	Ocation: 3470 12TH STREET, RIVERSIDE, CA 92501
Com	plete	d by:	N	EVILLE PEREIRA, P.E.	Date: 5/25/2015
					aic.
16.1 Very		IM w Seis		PIATE OCCUPANCY BASIC CONFIGURATION	ON CHECKLIST
Build	ding	Syste	m		
Gene	ral				
C N	k	N/A		building to the foundation. (Commentary: Sec. A	aplete, well-defined load path, including structural elements ial forces associated with the mass of all elements of the A.2.1.1. Tier 2: Sec. 5.4.1.1)
<u>C</u> .	JC	N/A	U	ADJACENT BUILDINGS: The clear distance be is greater than 4% of the height of the shorter be building types: W1, W1a, and W2. (Commentar	between the building being evaluated and any adjacent building uilding. This statement need not apply for the following y: Sec. A.2.1.2. Tier 2: Sec. 5.4.1.2)
CN	T.	N/A	Ü	MEZZANINES: Interior mezzanine levels are be to the seismic-force-resisting elements of the ma	raced independently from the main structure or are anchored ain structure. (Commentary: Sec. A.2.1.3. Tier 2: Sec. 5.4.1.3)
Build	ing	Config	urat	ion	·
C N	√	N/A)	WEAK STORY: The sum of the shear strengths direction shall not be less than 80% of the strengther 2: Sec. 5.4.2.1)	of the seismic-force-resisting system in any story in each gth in the adjacent story above. (Commentary: Sec. A.2.2.2.
C N	J	N/A	ال -		e-resisting system in any story shall not be less than 70% of adjacent story above or less than 80% of the average seismics above. (Commentary: Sec. A.2.2.3. Tier 2: Sec. 5.4.2.2)
C N	id_	N/A		VERTICAL IRREGULARITIES: All vertical ele the foundation. (Commentary: Sec. A.2.2.4. Tier	ements in the saismin force registing and
C N	c	N/A	U	GEOMETRY: There are no changes in the net be	orizontal dimension of the seismic-force-resisting system of
C N	С	N/A	U	MASS: There is no change in effective mass mo	re than 50% from one story to the next. Light roofs, cred. (Commentary: Sec. A.2.2.6. Tier 2: Sec. 5.4.2.5)
C N	C	N/A	U	TORSION: The estimated distance between the	story center of mass and the story center of rigidity is less nension. (Commentary: Sec. A.2.2.7. Tier 2: Sec. 5.4.2.6)
Low	Seisı	micity	: Co	mplete the Following Items in Additionto the I	tems for Very Low Seismicity.
Geolo	gic :	Site F	Iazaı	rds	• · · · · · · · · · · · · · · · · · · ·
C N	□	N/A)	LIQUEFACTION: Liquefaction-susceptible, satuseismic performance shall not exist in the foundard (Commentary: Sec. A.6.1.1. Tier 2: 5.4.3.1)	arated, loose granular soils that could jeopardize the building's ation soils at depths within 50 ft under the building.
C N		N/A)	SLOPE FAILURE: The building site is sufficient	ely remote from potential earthquake-induced slope failures or apable of accommodating any predicted movements without .1)
C N	4	N/A)		ture and surface displacement at the building also
Mode	rate	and]	High	Seismicity: Complete the Following Items in A	
Found	datio	n Co	nfigu	ration	
C N	d	N/A	}	OVERTURNING: The ratio of the least horizontal level to the building height (base/height) is greater	dimension of the seismic-force-resisting system at the foundation than $0.6S_a$. (Commentary: Sec. A.6.2.1. Tier 2: Sec. 5.4.3.3)
C N	~	NT/A	١.	Type (outside fin) is greater t	man 0.03 _a . (Commemary: Sec. A.6.2.1. Tier 2: Sec. 5.4.3.3)

TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C.

(Commentary: Sec. A.6.2.2. Tier 2: Sec. 5.4.3.4)

Lic. # : KW-06008839

Project Title: Engineer: Project Descr:

Project ID:

ASCE Seismic Base Shear

Printed: 1 JUN 2015, 11:09AM

File = c:\Users\Neville\DROPBO-1\UOBRECRD\201515283R-1\LEMONA-1\347012-1.EC6 ENERCALC, INC 1983-2015, Build:6.15.1.19, Ver.6.15.1.19

Licensee: Neville Pereira, P.E.

SEDAM CANALISM SEC RECEIVE Calculations per ASCE 7-10 Risk Category of Building or Other Structure: Seismic Importance Factor dd. d Sax Stealur. ASct Storage Max. Ground Motions, 5% Damping: Latitude = deg North g. 0.2 sec response Longitude = deg West g, 1.0 sec response Location: -Site Class. Site Coeff, and Design Category Site Classification "D": Shear Wave Velocity 600 to 1,200 ft/sec 4 No. 1 to 1 to 2 1 1 Site Coefficients Fa & Fv Fa BOOK AND THE BEST A (using straight-line interpolation from table values) 150 Maximum Considered Earthquake Acceleration S MS = Fa * Ss 1 500 **心**深点的 翻 计 [1] S M1 = Fv * S1 421-681-42 **Design Spectral Acceleration** S DS S MS 2/3 Lócal ASSLIE ECHILL S D1 S M1 2/3 ASSE 2 10 64 17 14 Seismic Design Category TIR DESIGNATION Resisting System A legistron with some rate with a Morrow Political about the Political action of the Complete Comment of the Complete Comment of the Comment Basic Seismic Force Resisting System . . . continue of the continue with Response Modification Coefficient "R" Poder South on System Overstrength Factor "Wo" Category "A & B" Limit: Category "C" Limit: 1,1 Deflection Amplification Factor * Cd * Category "D" Limit: Broken Gr NOTE! See ASCE 7-10 for all applicable foemoles. Category "E" Limit: Category "F" Limit: Redundancy Factor Seismic Design Category of D, E, or F therefore Redundancy Factor " p " = 1.3 Lateral Force Procedure **Equivalent Lateral Force Procedure** The Experience from Franchise is the grand according to be proposed by ASCE ATORIES Structure Type for Building Period Calculation: " Ct " value " hn ": Height from base to highest level = in ft " x " value "Ta " Approximate fundemental period using Eq. 12.8-7: talket production sec "TL": Long-period transition period per ASCE 7-10 Maps 22-12 -> 22-16 Building Period "Ta" Calculated from Approximate Method selected sec Li Responsa Coatriciere S DS: Short Period Design Spectral Response From Eq. 12.8-2, Preliminary Cs " R ": Response Modification Factor From Eq. 12.8-3 & 12.8-4, Cs need not exceed "I": Seismic Importance Factor From Eq. 12.8-5 & 12.8-6, Cs not be less than User has selected ASCE 12.8.1.3: Regular structure, Less than 5 Stories and with T <<= 0.5 sec, SO Ss <= 1.5 for Cs calculation

Project Title: Engineer: Project Descr:

Project ID:

ASCE Seismic Base Shear

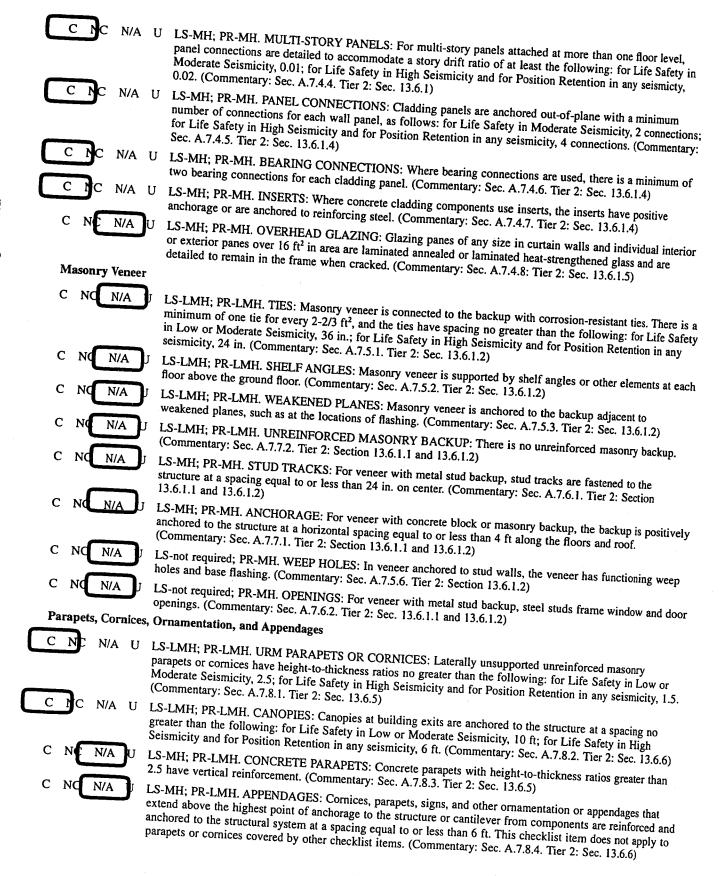
Printed: 1 JUN 2015 11:09AM
File = c:\Users\Neville\DROPBO-1\UOBRECRD\2015\15283R-1\LEMONA-1\347012-1.EC6
ENERCALC, INC. 1983-2015, Build:6.15.1.19, Ver:6.15.1.19

Cs =	0.3333 from 12.8.1.1		W (se	e Sum W	/i below) =	4.34 k	+ 7 1
			Seismic Base SI	hear V	= Cs • W =	k	
	indice of sections in the force.						
k " : hx exponent i able of building We	based on Ta = lights by Floor Level						
Level #	Wi : Weight	Hi : Height	(Wi * Hi) *k	Cvx	Fx=Cvx * V	Sum Story Shear	Sum Story Momen
Sum Wi =) (k	Sum Wi * Hi =	k-ft		Total Base Shear =	k Base Moment =	k-ft
hdo kuga ka	o - Seismic Designic an go	ry Burr					4 / 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Level #	Wi	Fi	Sum Fi		Sum Wi	Fpx	
Fi		teral Force applied	at the level.		attached to it.		-

Project: 3420 & 34	Location: 3420 & 3470 12TH STREET, RIVERSIDE, CA 92501
Completed by: N	EVILLE PEREIRA, P.E. Date: 5/25/2015
	UCTURAL CHECKLIST
Life Safety System	ns
C NC N/A U	LS-LMH; PR-LMH. FIRE SUPPRESSION PIPING: Fire suppression piping is anchored and braced in accordance with NFPA-13. (Commentary: Sec. A.7.13.1. Tier 2: Sec. 13.7.4)
C NC N/A U	with NFPA-13. (Commentary: Sec. A.7.13.2. Tier 2: Sec. 13.7.4)
	LS-LMH; PR-LMH. EMERGENCY POWER: Equipment used to power or control life safety systems is anchored or braced. (Commentary: Sec. A.7.12.1. Tier 2: Sec. 13.7.7)
	LS-LMH; PR-LMH. STAIR AND SMOKE DUCTS: Stair pressurization and smoke control ducts are braced and have flexible connections at seismic joints. (Commentary: Sec. A.7.14.1. Tier 2: Sec. 13.7.6)
C NC NA U	
C NC N/A U	LS-not required; PR-LMH. EMERGENCY LIGHTING: Emergency and egress lighting equipment is anchored or braced. (Commentary: Sec. A.7.3.1. Tier 2: Sec. 13.7.9)
Hazardous Mater	ials
C N N/A U	LS-LMH; PR-LMH. HAZARDOUS MATERIAL EQUIPMENT: Equipment mounted on vibration isolators and containing hazardous material is equipped with restraints or snubbers. (Commentary: Sec. A.7.12.2. Tier 2: 13.7.1)
C N N/A U	LS-LMH; PR-LMH. HAZARDOUS MATERIAL STORAGE: Breakable containers that hold hazardous material, including gas cylinders, are restrained by latched doors, shelf lips, wires, or other methods. (Commentary: Sec. A.7.15.1. Tier 2: Sec. 13.8.4)
C NO N/A	LS-MH; PR-MH. HAZARDOUS MATERIAL DISTRIBUTION: Piping or ductwork conveying hazardous materials is braced or otherwise protected from damage that would allow hazardous material release. (Commentary: Sec. A.7.13.4. Tier 2: Sec. 13.7.3 and 13.7.5)
C NC N/A U	LS-MH; PR-MH. SHUT-OFF VALVES: Piping containing hazardous material, including natural gas, has shut-off valves or other devices to limit spills or leaks. (Commentary: Sec. A.7.13.3. Tier 2: Sec. 13.7.3 and 13.7.5)
C NO N/A U	LS-LMH; PR-LMH. FLEXIBLE COUPLINGS: Hazardous material ductwork and piping, including natural gas piping, has flexible couplings. (Commentary: Sec. A.7.15.4, Tier 2: Sec.13.7.3 and 13.7.5)
C NO N/A J	LS-MH; PR-MH. PIPING OR DUCTS CROSSING SEISMIC JOINTS: Piping or ductwork carrying hazardous material that either crosses seismic joints or isolation planes or is connected to independent structures has couplings or other details to accommodate the relative seismic displacements. (Commentary: Sec. A.7.13.6. Tier 2: Sec.13.7.3, 13.7.5, and 13.7.6)
Partitions	
C NON/A	LS-LMH; PR-LMH. UNREINFORCED MASONRY: Unreinforced masonry or hollow-clay tile partitions are braced at a spacing of at most 10 ft in Low or Moderate Seismicity, or at most 6 ft in High Seismicity. (Commentary: Sec. A.7.1.1. Tier 2: Sec. 13.6.2)
C N(N/A)	LS-LMH; PR-LMH. HEAVY PARTITIONS SUPPORTED BY CEILINGS: The tops of masonry or hollow-clay tile partitions are not laterally supported by an integrated ceiling system. (Commentary: Sec. A.7.2.1. Tier 2: Sec. 13.6.2)
C NO N/A	LS-MH; PR-MH. DRIFT: Rigid cementitious partitions are detailed to accommodate the following drift ratios: in steel moment frame, concrete moment frame, and wood frame buildings, 0.02; in other buildings, 0.005.

(Commentary A.7.1.2 Tier 2: Sec. 13.6.2)

С	i c i	N/A	U	LS-not required; PR-MH. LIGHT PARTITIONS SUPPORTED BY CEILINGS: The tops of gypsum board partitions are not laterally supported by an integrated ceiling system. (Commentary: Sec. A.7.2.1. Tier 2:
C	•	N.A	U	LS-not required; PR-MH. STRUCTURAL SEPARATIONS: Partitions that cross structural separations have seismic or control joints. (Commentary: Sec. A 7.1.3. Tier 2. Sec. 13.6.2)
C		N/A	U	LS-not required; PR-MH. TOPS: The tops of ceiling-high framed or panelized partitions have lateral bracing to the structure at a spacing equal to or less than 6 ft. (Commentary: Sec. A.7.1.4. Tier 2. Sec. 13.6.2)
Ceili	ngs			
Ci	C 1	N/A	U	LS-MH; PR-LMH. SUSPENDED LATH AND PLASTER: Suspended lath and plaster ceilings have attachments that resist seismic forces for every 12 ft ² of area. (Commentary: Sec. A.7.2.3. Tier 2: Sec. 13.6.4)
	, 	V/A	ָּט ה.	that resist seismic forces for every 12 ft ² of area. (Commentary: Sec. A 7.2.3 Tier 2: Sec. 13.6.4)
	, <u> </u>	N/A	J ⁾	greater than 144 ft ² , and ceilings of smaller areas that are not surrounded by restraining partitions, are laterally location has a minimum of four diagonal wires and compression struts, or diagonal members capable of resisting compression. (Commentary: Sec. A.7.2.2, Tier 2: Sec. 13.6.4)
	C N	I/A	U	LS-not required; PR-MH. EDGE CLEARANCE: The free edges of integrated suspended ceilings with continuous areas greater than 144 ft ² have clearances from the enclosing wall or partition of at least the following: in Moderate Seismicity, 1/2 in.; in High Seismicity, 3/4 in. (Commentary: Sec. A.7.2.4. Tier 2: Sec. 13.6.4)
C N	C N	I/A	U	LS-not required; PR-MH. CONTINUITY ACROSS STRUCTURE JOINTS: The ceiling system does not cross any seismic joint and is not attached to multiple independent structures. (Commentary: Sec. A.7.2.5. Tier 2: Sec. 13.6.4)
C N	Id N	I/A)	LS-not required; PR-H. EDGE SUPPORT: The free edges of integrated suspended ceilings with continuous areas greater than 144 ft ² are supported by closure angles or channels not less than 2 in. wide. (Commentary: Sec. A.7.2.6. Tier 2: Sec. 13.6.4)
C N	C N	/A	U	LS-not required; PR-H. SEISMIC JOINTS: Acoustical tile or lay-in panel ceilings have seismic separation joints such that each continuous portion of the ceiling is no more than 2500 ft ² and has a ratio of long-to-short dimension no more than 4-to-1. (Commentary: Sec. A.7.2.7. Tier 2: 13.6.4)
Light	Fixtu	res		
C N	_	/A	þ	LS-MH; PR-MH. INDEPENDENT SUPPORT: Light fixtures that weigh more per square foot than the ceiling they penetrate are supported independent of the grid ceiling suspension system by a minimum of two wires at diagonally opposite corners of each fixture. (Commentary: Sec. A.7.3.2. Tier 2: Sec. 13.6.4 and 13.7.9)
CN	C N	/A	•	LS-not required; PR-H. PENDANT SUPPORTS: Light fixtures on pendant supports are attached at a spacing equal to or less than 6 ft and, if rigidly supported, are free to move with the structure to which they are attached without damaging adjoining components. (Commentary: A.7.3.3. Tier 2: Sec. 13.7.9)
C N	C N	/A	U	LS-not required; PR-H. LENS COVERS: Lens covers on light fixtures are attached with safety devices. (Commentary: Sec. A.7.3.4. Tier 2: Sec. 13.7.9)
Cladd	ing ar	nd G		
	C N	'A 1		LS-MH; PR-MH. CLADDING ANCHORS: Cladding components weighing more than 10 lb/ft ² are mechanically anchored to the structure at a spacing equal to or less than the following: for Life Safety in Moderate Seismicity, 6 ft; for Life Safety in High Seismicity and for Position Retention in any seismicity, 4 ft. (Commentary: Sec. A.7.4.1. Tier 2: Sec. 13.6.1)
C N	C N/	'A 1	U	LS-MH; PR-MH. CLADDING ISOLATION: For steel or concrete moment frame buildings, panel connections are detailed to accommodate a story drift ratio of at least the following: for Life Safety in Moderate Seismicity, 0.01; for Life Safety in High Seismicity and for Position Retention in any seismicity, 0.02. (Commentary: Sec. A.7.4.3. Tier 2: Section 13.6.1)



Masonry Chimneys N/A LS-LMH; PR-LMH. URM CHIMNEYS: Unreinforced masonry chimneys extend above the roof surface no more than the following: for Life Safety in Low or Moderate Seismicity, 3 times the least dimension of the chimney; for Life Safety in High Seismicity and for Position Retention in any seismicity, 2 times the least dimension of the chimney. (Commentary: Sec. A.7.9.1. Tier 2: 13.6.7) N/A LS-LMH; PR-LMH. ANCHORAGE: Masonry chimneys are anchored at each floor level, at the topmost ceiling level, and at the roof. (Commentary: Sec. A.7.9.2. Tier 2: 13.6.7) Stairs LS-LMH; PR-LMH. STAIR ENCLOSURES: Hollow-clay tile or unreinforced masonry walls around stair N/A enclosures are restrained out-of-plane and have height-to-thickness ratios not greater than the following: for Life Safety in Low or Moderate Seismicity, 15-to-1; for Life Safety in High Seismicity and for Position Retention in any seismicity, 12-to-1. (Commentary: Sec. A.7.10.1. Tier 2: Sec. 13.6.2 and 13.6.8) NC -LMH; PR-LMH. STAIR DETAILS: In moment frame structures, the connection between the stairs and the structure does not rely on shallow anchors in concrete. Alternatively, the stair details are capable of accommodating the drift calculated using the Quick Check procedure of Section 4.5.3.1 without including any lateral stiffness contribution from the stairs. (Commentary: Sec. A.7.10.2. Tier 2: 13.6.8) Contents and Furnishings LS-MH; PR-MH. INDUSTRIAL STORAGE RACKS: Industrial storage racks or pallet racks more than 12 ft N/A high meet the requirements of ANSI/MH 16.1 as modified by ASCE 7 Chapter 15. (Commentary: Sec. N/A U LS-H; PR-MH. TALL NARROW CONTENTS: Contents more than 6 ft high with a height-to-depth or heightto-width ratio greater than 3-to-1 are anchored to the structure or to each other. (Commentary: Sec. A.7.11.2. Tier 2: Sec. 13.8.2) N/A U LS-H; PR-H. FALL-PRONE CONTENTS: Equipment, stored items, or other contents weighing more than 20 lb whose center of mass is more than 4 ft above the adjacent floor level are braced or otherwise restrained. (Commentary: Sec. A.7.11.3. Tier 2: Sec. 13.8.2) LS-not required; PR-MH. ACCESS FLOORS: Access floors more than 9 in. high are braced. (Commentary: N/A N/A LS-not required; PR-MH. EQUIPMENT ON ACCESS FLOORS: Equipment and other contents supported by access floor systems are anchored or braced to the structure independent of the access floor. (Commentary: LS-not required; PR-H. SUSPENDED CONTENTS: Items suspended without lateral bracing are free to swing N/A U from or move with the structure from which they are suspended without damaging themselves or adjoining components. (Commentary. A.7.11.6. Tier 2: Sec. 13.8.2) Mechanical and Electrical Equipment LS-H; PR-H. FALL-PRONE EQUIPMENT: Equipment weighing more than 20 lb whose center of mass is more than 4 ft above the adjacent floor level, and which is not in-line equipment, is braced. (Commentary: N/A U LS-H; PR-H. IN-LINE EQUIPMENT: Equipment installed in-line with a duct or piping system, with an operating weight more than 75 lb, is supported and laterally braced independent of the duct or piping system.

IS-H; PR-MH. TALL NARROW EQUIPMENT: Equipment more than 6 ft high with a height-to-depth or height-to-width ratio greater than 3-to-1 is anchored to the floor slab or adjacent structural walls.

LS-not required; PR-MH. MECHANICAL DOORS: Mechanically operated doors are detailed to operate at a

(Commentary: Sec. A.7.12.5. Tier 2: Sec. 13.7.1)

(Commentary: Sec. A.7.12.6. Tier 2: Sec. 13.7.1 and 13.7.7)

story drift ratio of 0.01. (Commentary: Sec. A.7.12.7. Tier 2: Sec. 13.6.9)

NC

N/A

